

REVISED
4/29/25
5/18/25
5/12/25
6/2/25
6/8/25
6/24/25

NOTE: PROVIDE PROPANE GAS HOOKUP ON PATIO

APPROXIMATE 13'X24'
STONE PATIO TO
MATCH EXISTING @
GRADE

EXISTING STONE PATIO

24' EXISTING RAMP

22'-4" NEW FOUNDATION
ALIGN W/ EXTS.

BASEMENT

8" MIN. POURED
CONCRETE
WALL

LOW SLAB @ GRADE
BUILD UP STAIRS & LANDING
WITH MASONRY

WORK
BENCH
AREA

FILL IN OPNS

REMOVE WALLS

DEMO EXISTING CONCRETE
SLAB & EXCAVATE A
MIN. 4' DOWN, POUR 2" MIN.
BEST SLAB ON 6 MIL POLY V.B.
& 5" LAYER OF 3/4" CRUSHED
STONES & 2" RIGID FOAM INSUL.

BASEMENT AREA

SAVCLIT CONC. SLAB @ WALL OF
STAIRS & MUDROOM

NEW 8" POURED CONC.
WALL 15" FROM EXTS.
ON 10" V.V. X 8" H KEYED
CONC. FTG.

EXISTING SLAB TO REMAIN

UTILITY AREA
FURNACE, HOT WATER HR.
ON DEMAND HEATERS
VENT THEY WALL

IRON RAILING
4'-0" E 4'-0" F
2'-0" 4'-0" 4'-0"
STONE LANDING
7" HIGH
CLUBS
COPPER CRICKET
SWITCH TO SECOND FLOOR
SPOTLIGHTS
ALIGN

NEW EXISTING
ALIGN

ALIGN

LIGHT IN HALL

PROVIDE 220V.
OUTLET FOR
CUPBOARD ELECTRICAL
CAP. VENT (V.V.)
DRAINER

POST SCHEDULE

MARK	MINIMUM SIZES	TOP CONNECTION
P1	4x4 or 2-2x6 or 2-2x4	4 - 10d TOE NAILS OR AS SHOWN ON DRAWINGS
P2	4x6 or 2-2x6 or 3-2x4	6 - 10d TOE NAILS OR AS SHOWN ON DRAWINGS
P3	6x6 or 3-2x6 or 5-2x4	2 - SIMPSON "AS5" FRAMING ANCHORS OR AS SHOWN ON DRAWINGS
P4	8x8 or 7x7 or 6-2x6 or 5 1/4" x 5 1/4" PSL	4 - SIMPSON "AS5" FRAMING ANCHORS OR AS SHOWN ON DRAWINGS
P5	8x8 or 6-2x6 or 5 1/4" x 7 1/4" PSL	4 - SIMPSON "AS5" FRAMING ANCHORS OR AS SHOWN ON DRAWINGS

JACK & KING STUDS ARE CONSIDERED PART OF HEADER POSTS.
PSL = PARALLEL BY TRUSSJOIST OR APPROVED EQUAL.
POSTS SHALL REST ON GINDER, FOUNDATION OR APPROVED SOLID BLOCKING.
ALL OTHER POSTS NOT SHOWN ON PLAN SHALL BE 2-2x4 MIN.

FOUNDATION PLAN

REFER TO S1 FOR FOUNDATION DETAILS

S drawings are for Structural information only.
Refer to Sheet S5 for the beam, joists and header
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WALL LEGEND

=====	EXISTING
-----	EXTS. TO BE REMOVED
=====	EXTG. OPENING TO BE FILLED
=====	PROPOSED

FIRST FLOOR PLAN

ELECTRICAL OUTLETS PER CODE, LIGHTING & SWITCHING LOCATIONS FOR PRICING.
CONTRACTOR & ELECTRICIAN TO "WALKTHROUGH" WITH OWNERS TO FINALIZE PLAN.

DRAWN BY
ML

CHECKED BY

APPROVED BY

FILE NAME

DATE
4/24/25

MICHAEL LOBUGLIO A.I.A ARCHITECT

62 TAUNTON LAKE RD., NEWTOWN, CT
TEL. (203) 512-0353

CARVALHO RESIDENCE
172 NORTH SALEM ROAD
RIDGEFIELD, CT

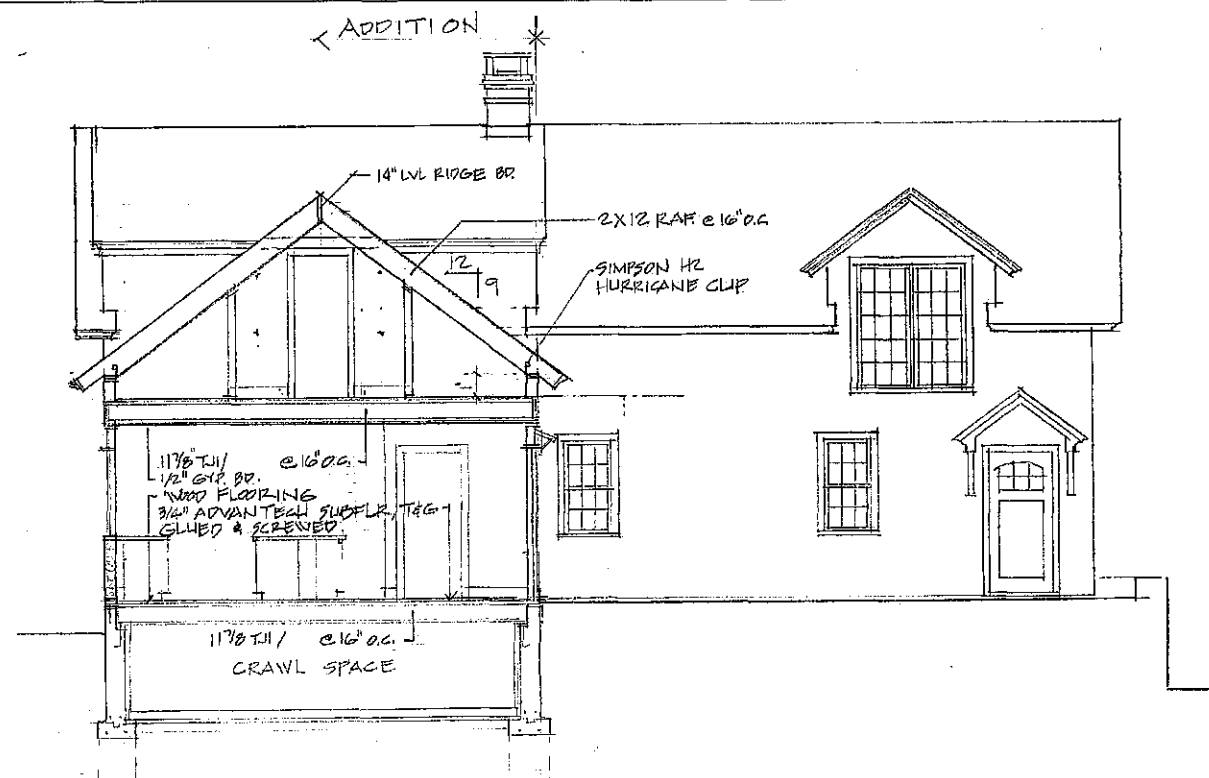
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A-1

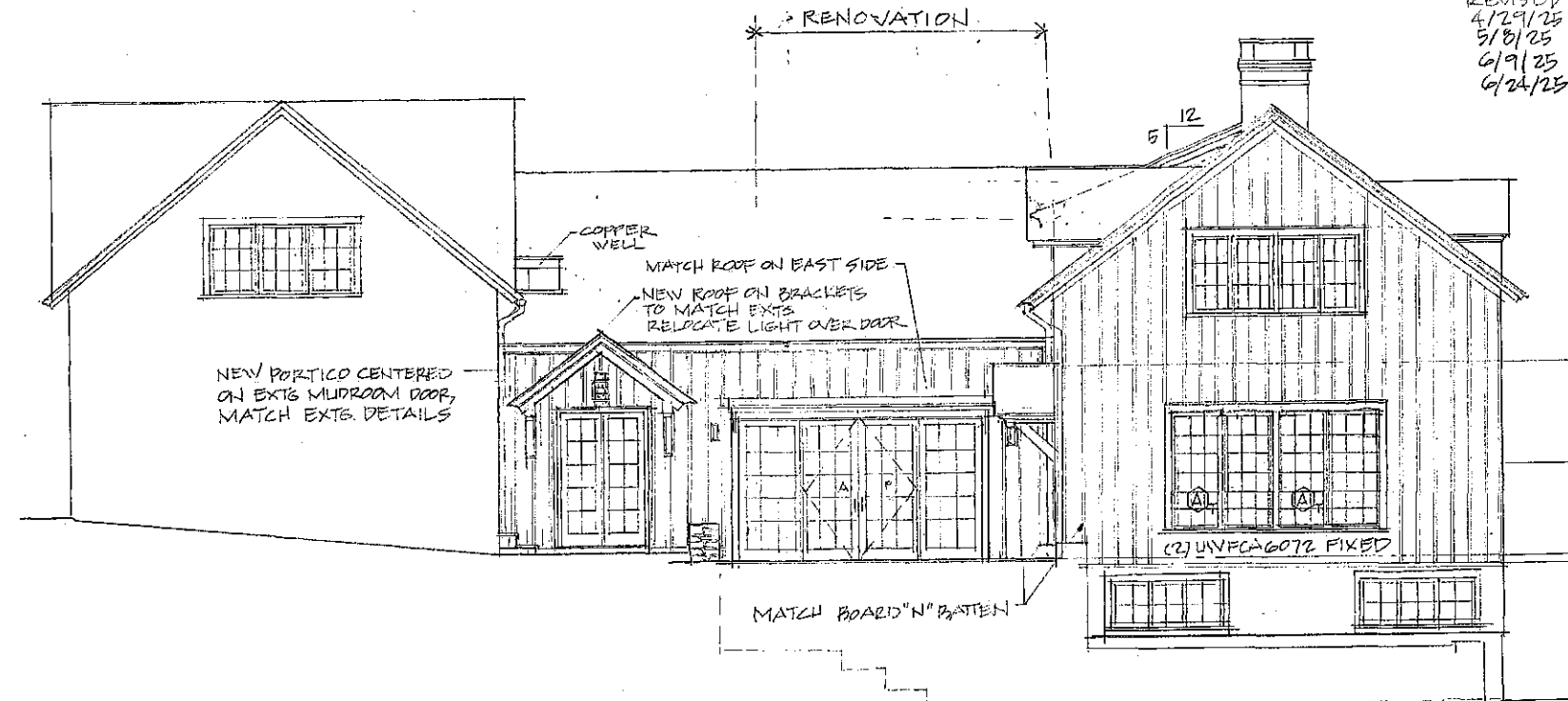
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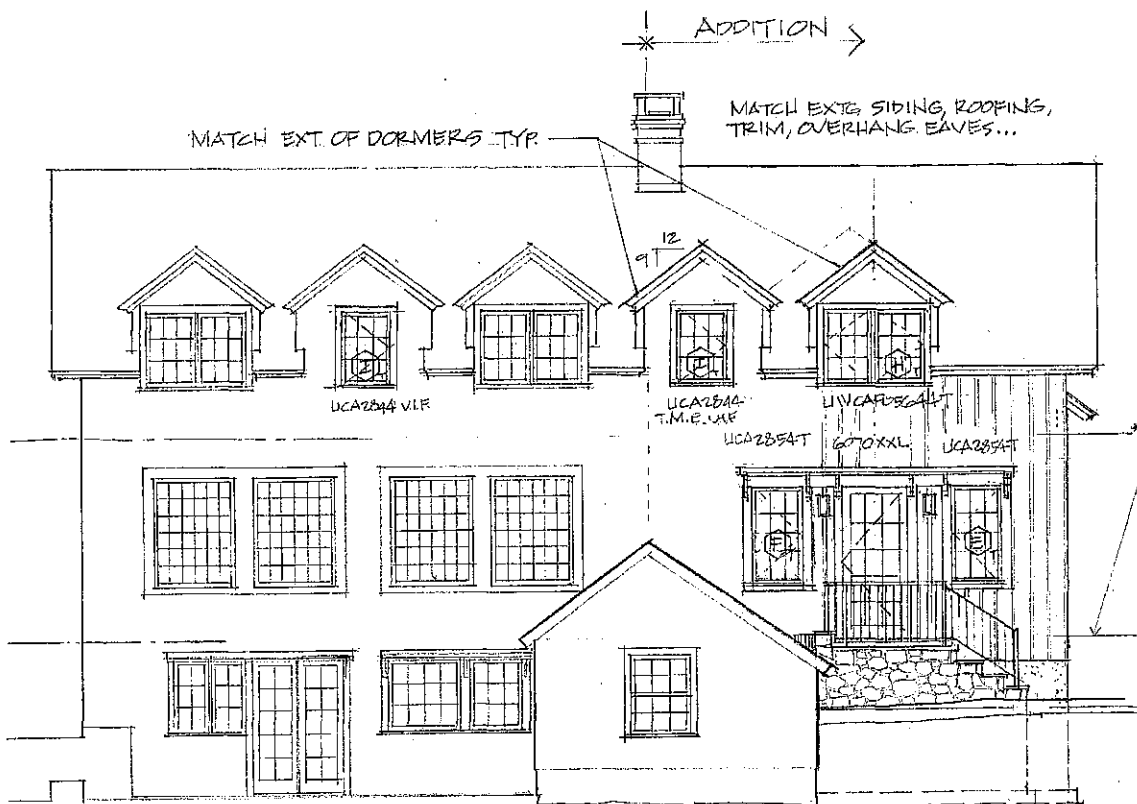
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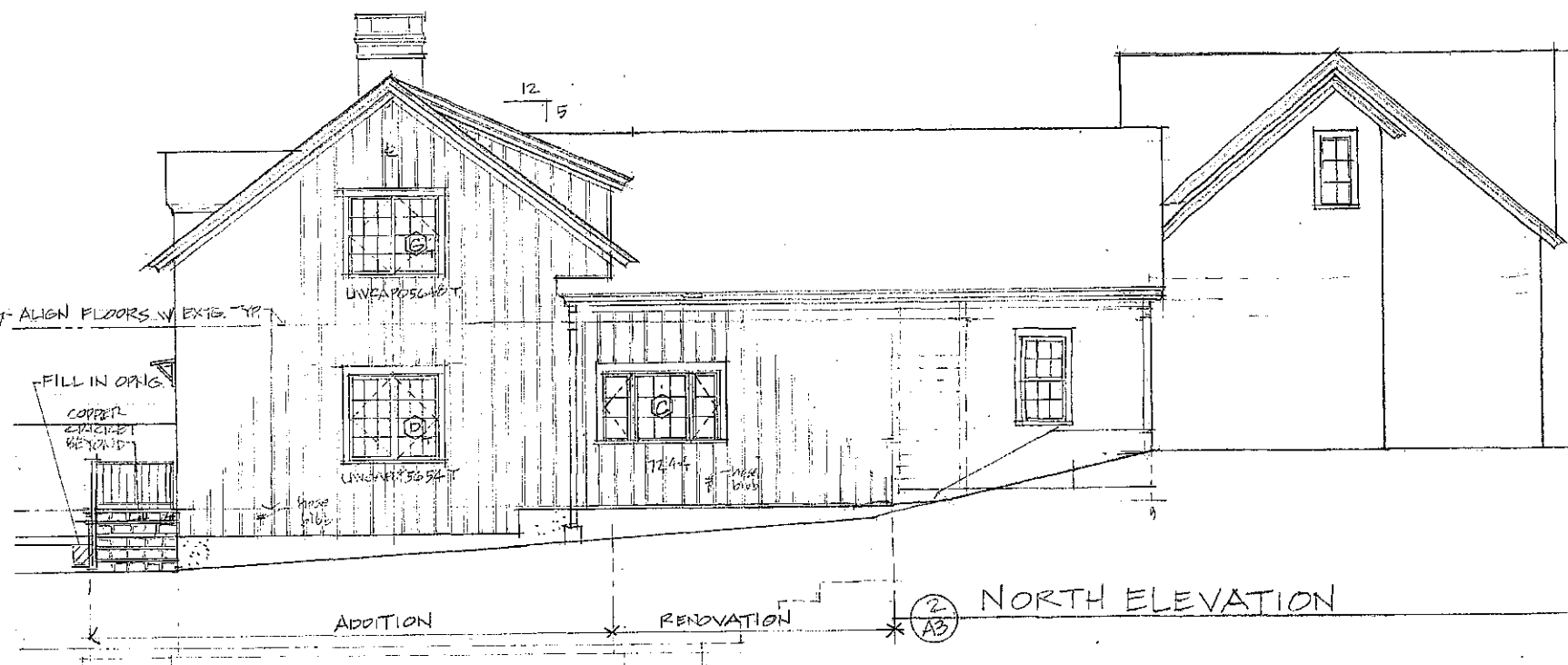
③ WEST ELEVATION
A3



④ SOUTH ELEVATION
A3



① EAST ELEVATION
A3



② NORTH ELEVATION
A3

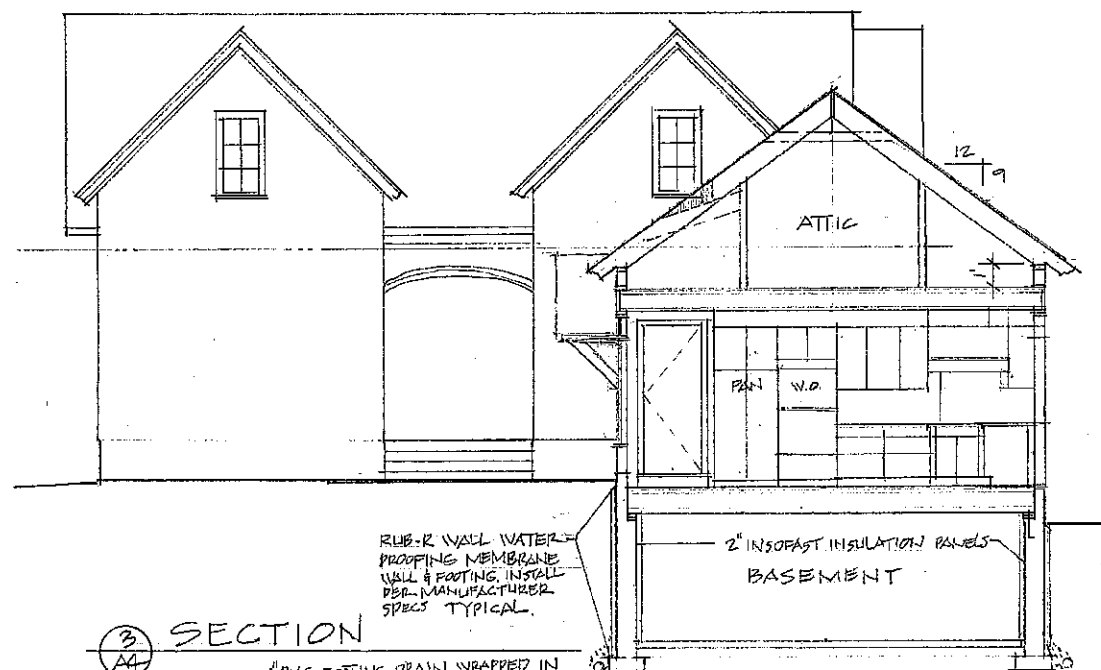
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CONVERT WINDOWS & DOOR SIZES TO JELD-WEN SPECIFICATIONS THAT MATCH EXISTING, VERIFY IN FIELD

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62 TAUNTON LAKE RD., NEWTOWN, CT TEL. (203) 512-0353			SCALE: 1/4" = 1'-0"	

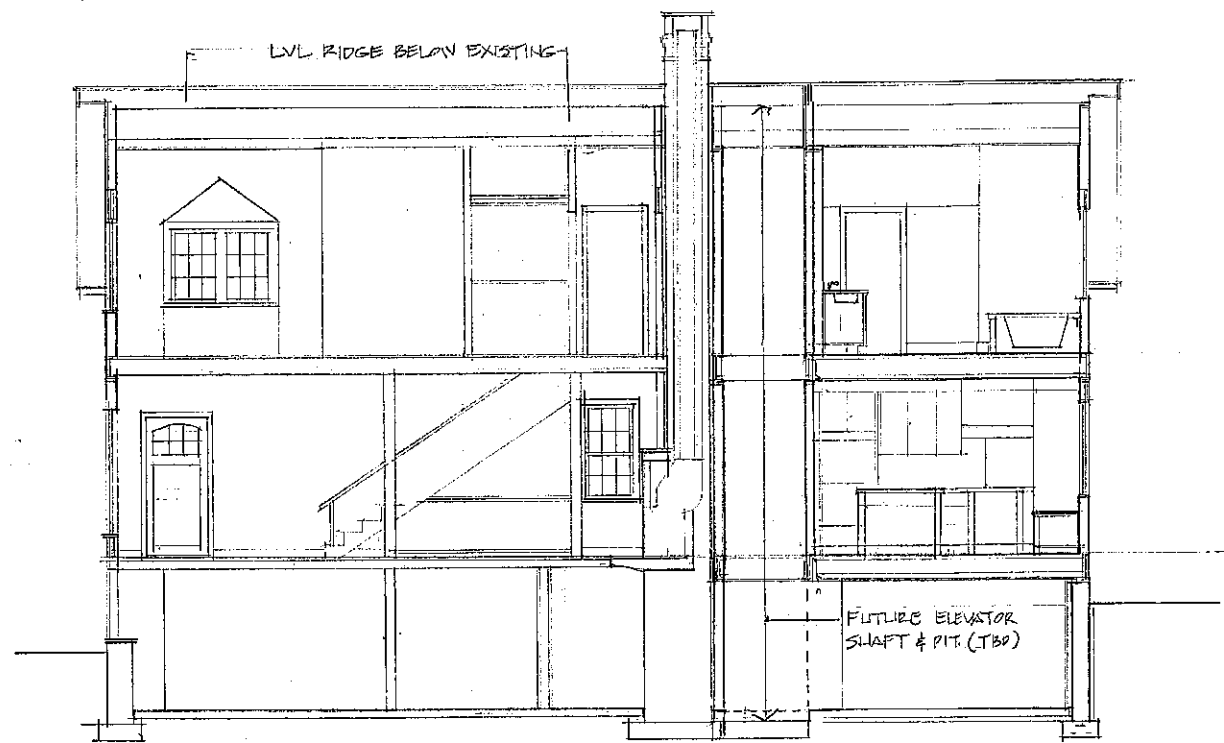
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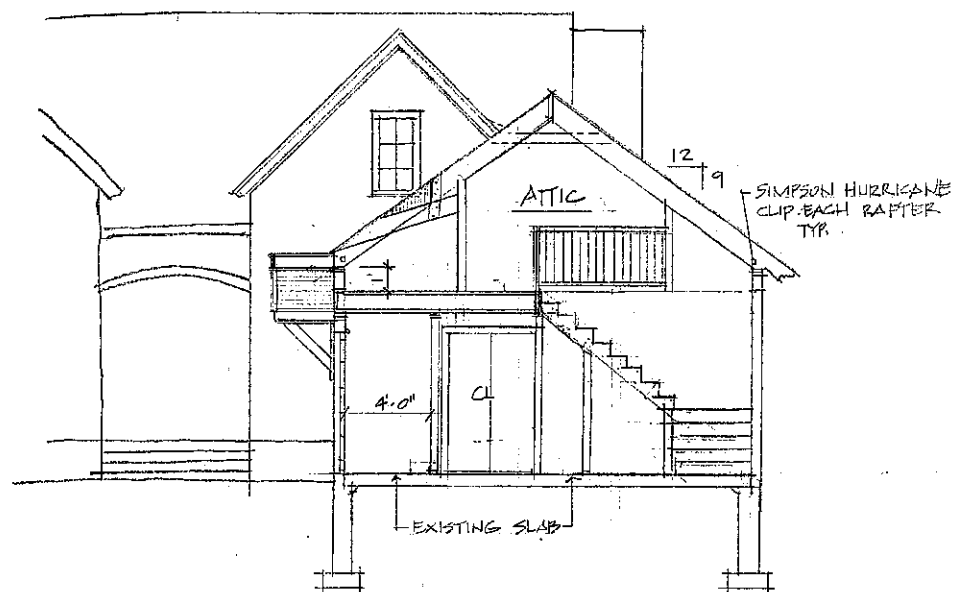


③ SECTION
A4

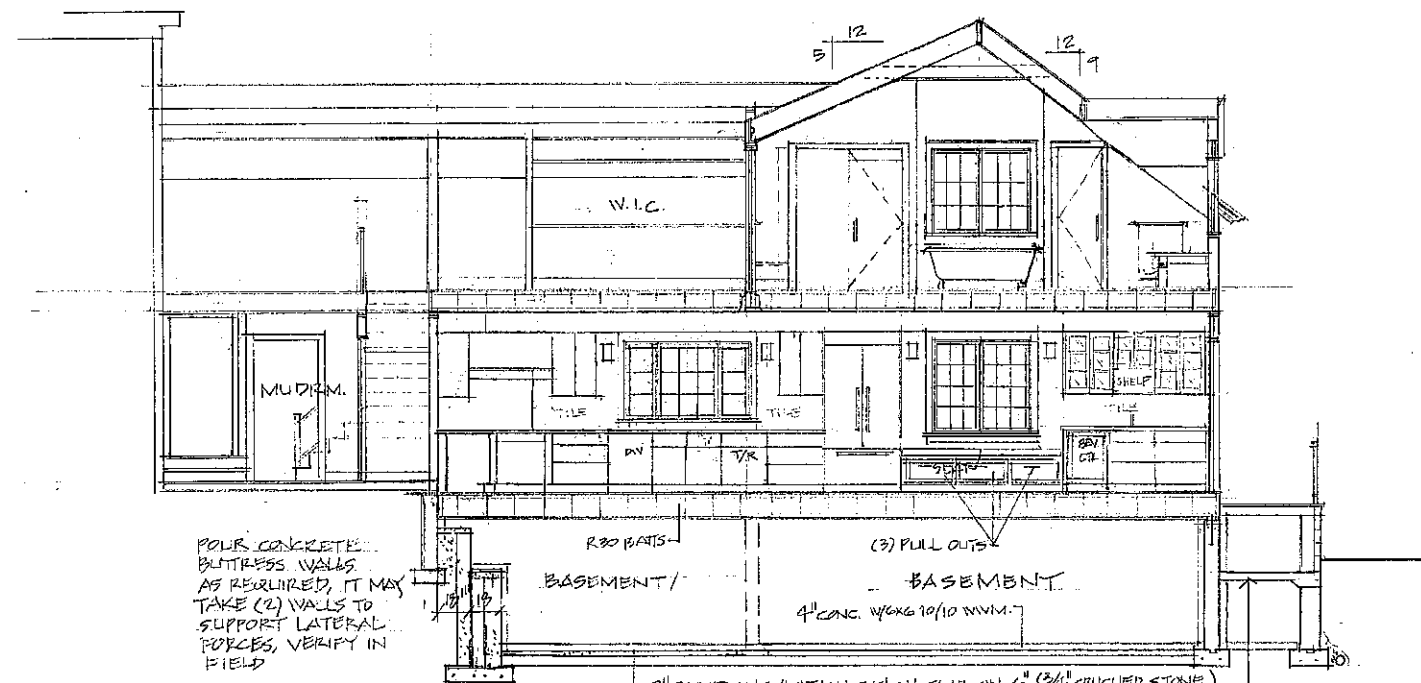
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④ SECTION
A4



① SECTION
A4



② SECTION
A4

LOW SLAB, BUILD UP LANDING & STEPS W/ MASONRY

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MICHAEL LOBUGLIO A.I.A ARCHITECT			CARVALHO RESIDENCE 172 NORTH SALEM ROAD RIDGEFIELD, CT	
62 TAUNTON LAKE RD., NEWTOWN, CT TEL. (203) 512-0353			SCALE: 1/4" = 1'-0"	

A-4

FOUNDATION WALL

16" THICK TOTAL
REINF. W/ 2-#5 CONT. TOP
AND AT MID HEIGHT
AND
#4 @ 18" O/C VERT. &
#4 @ 24" O/C HORIZ. INTERIOR
FACE TYPICAL

#4 X 24" @ 36" O/C DOWELS
FOOTING TO WALL
ON
12" X 22" CONC. FTG
WITH 2-#5 CONT. BOTTOM

IN

EX

FOUNDATION WALL

10" THICK TOTAL
#4 @ 16" O/C VERT. & #4 @ 24" O/C HORIZ. INTERIOR
AND AT MID HEIGHT
1- #4 CONT. IN STEM
#4 @ 16" O/C VERT. &
#4 @ 24" O/C HORIZ. INTERIOR
FACE TYPICAL
#4 X 24" @ 36" O/C DOWELS
FOOTING TO WALL
ON
12" X 22" CONC. FTG
WITH 2- #5 CONT. BOTTOM

IN OUT

SLAB

FOUNDATION WALL
10" THICK TOTAL
REIN. W/ 2#5 CONT. TOP
AND MID HEIGHT
&
#4 @ 18" O/C VERT. &
#4 @ 24" O/C HORIZ. INTERIOR
FACE TYPICAL

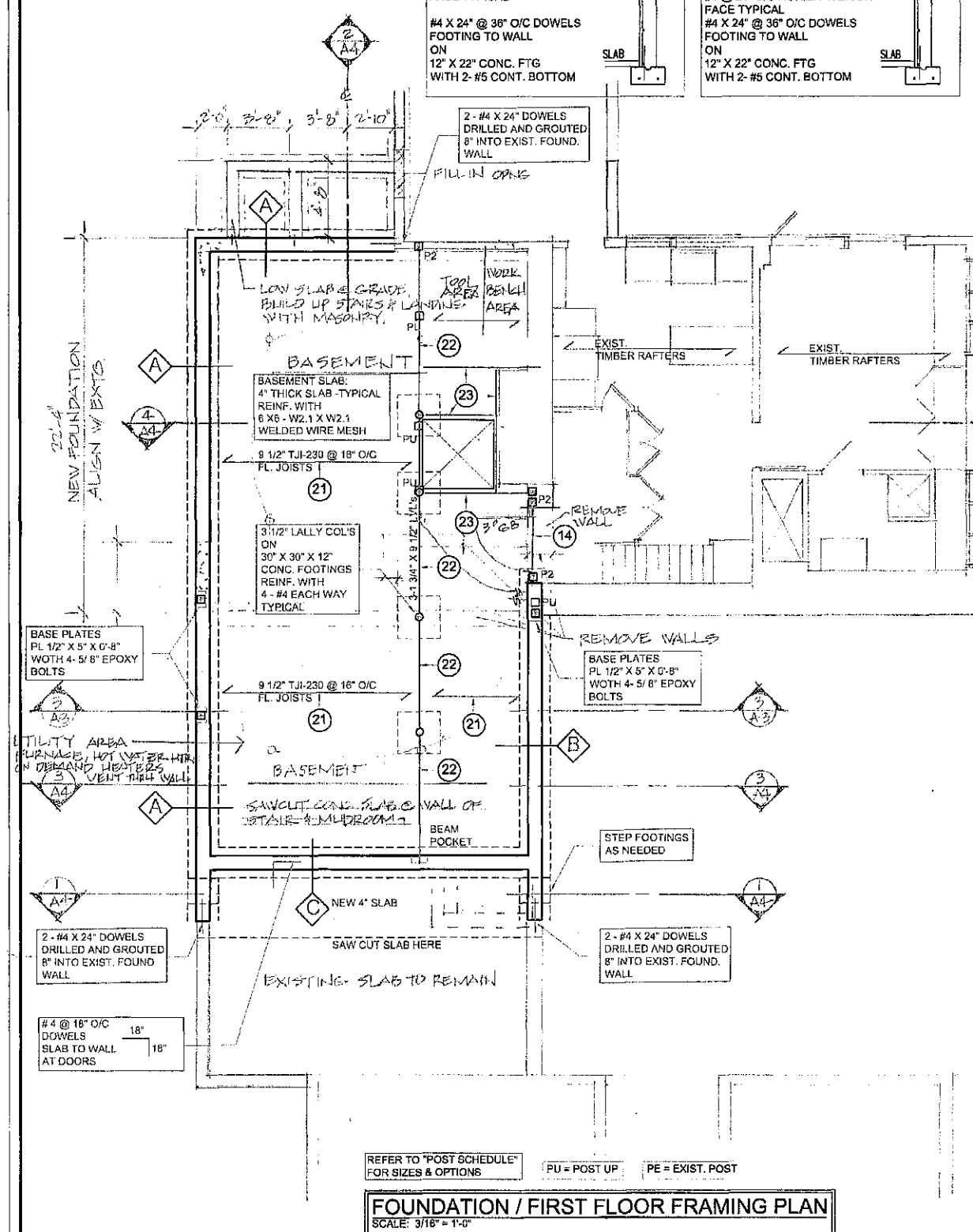
#4 X 24" @ 36" O/C DOWELS
FOOTING TO WALL
ON
12" X 22" CONC. FTG
WITH 2-#5 CONT. BOTTOM

SLAB

IN

OUT

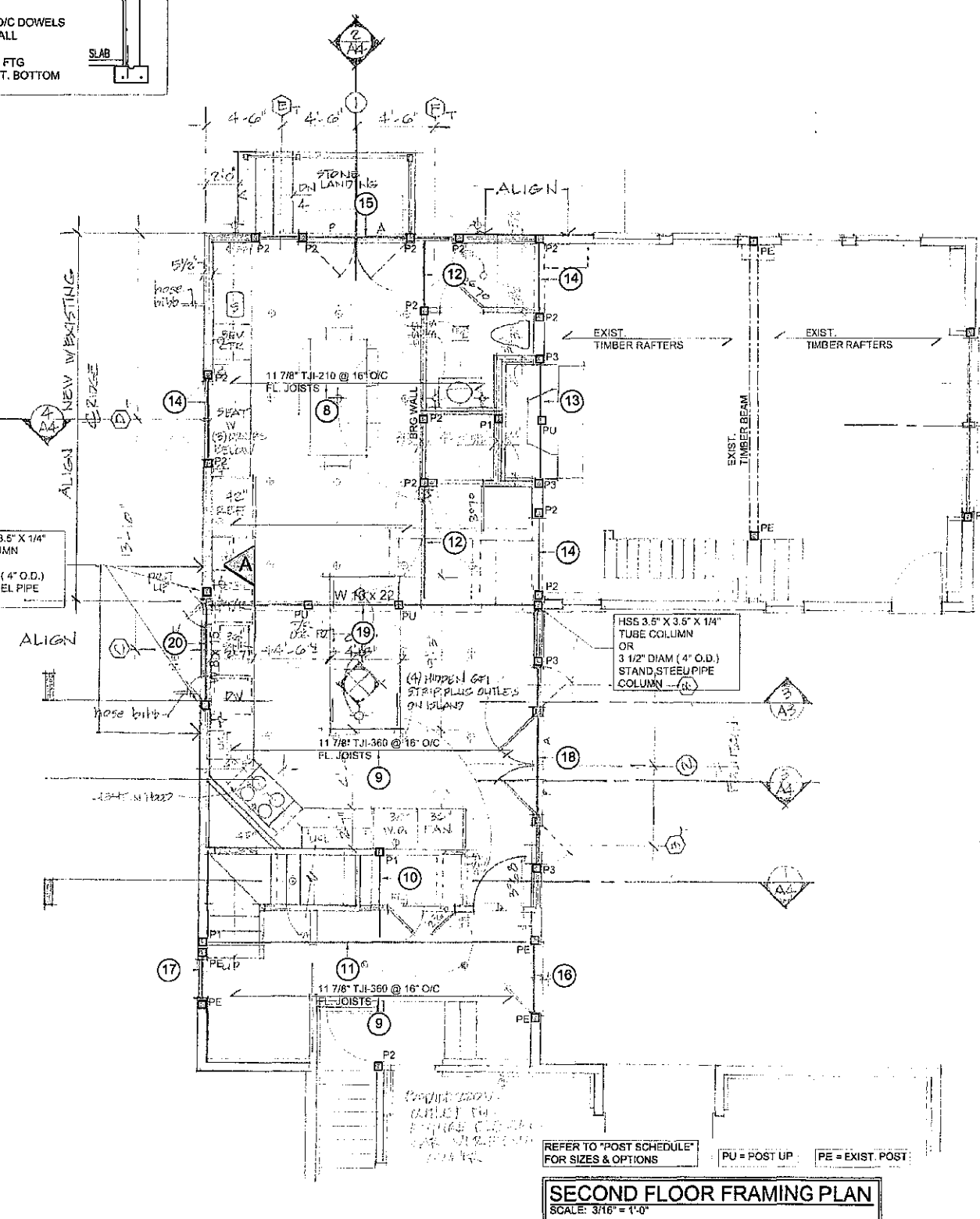
SLAB



FOUNDATION / FIRST FLOOR FRAMING PLAN
SCALE: 3/16" = 1'-0"

FOUNDATION PLAN

PROVIDE STRUCTURAL STEEL
SHOP DRAWINGS FOR
APPROVAL BY THE STRUCTURAL
ENGINEER BEFORE
FABRICATION.



SECOND FLOOR FRAMING PLAN
SCALE: 3/16" = 1'-0"

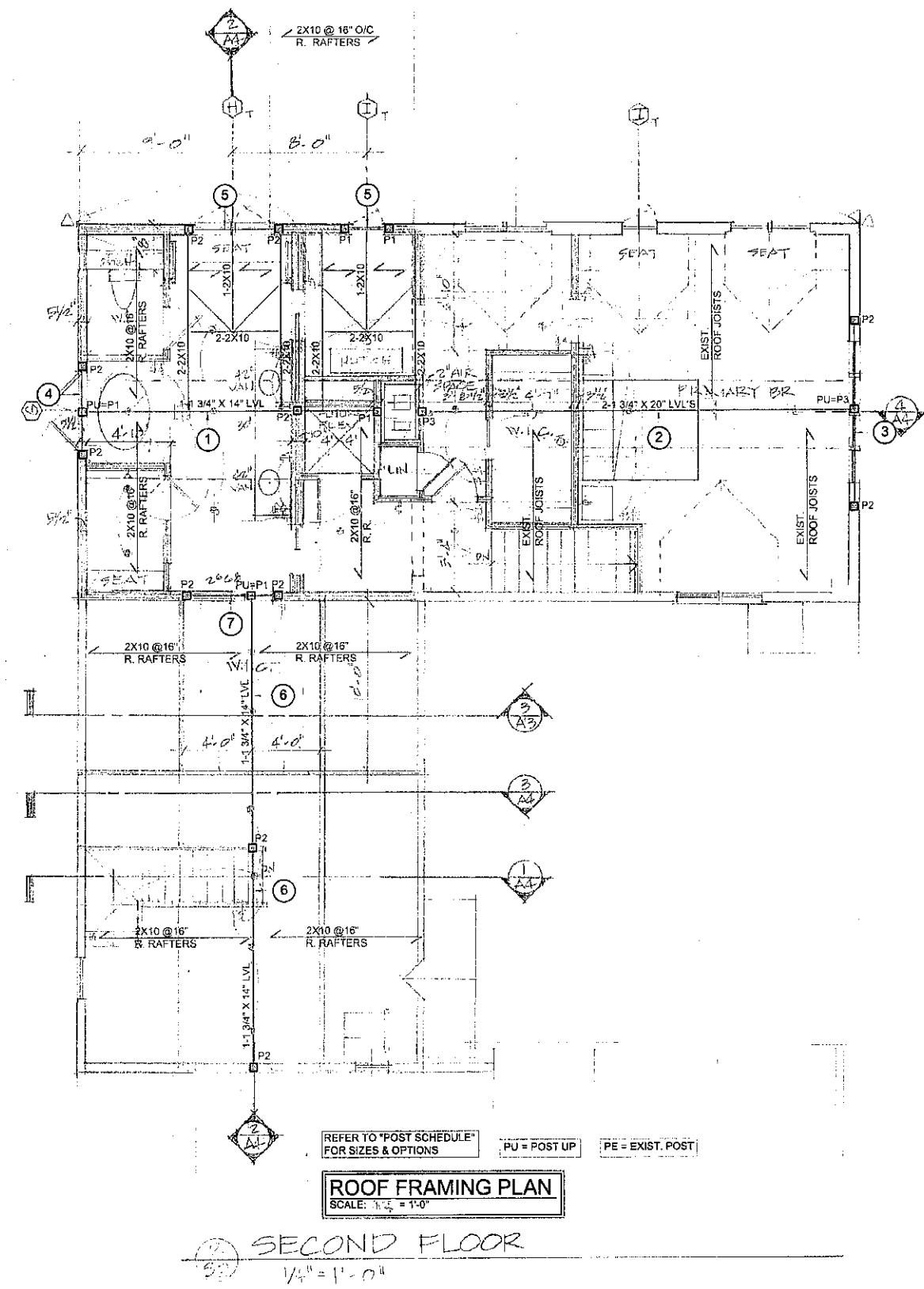
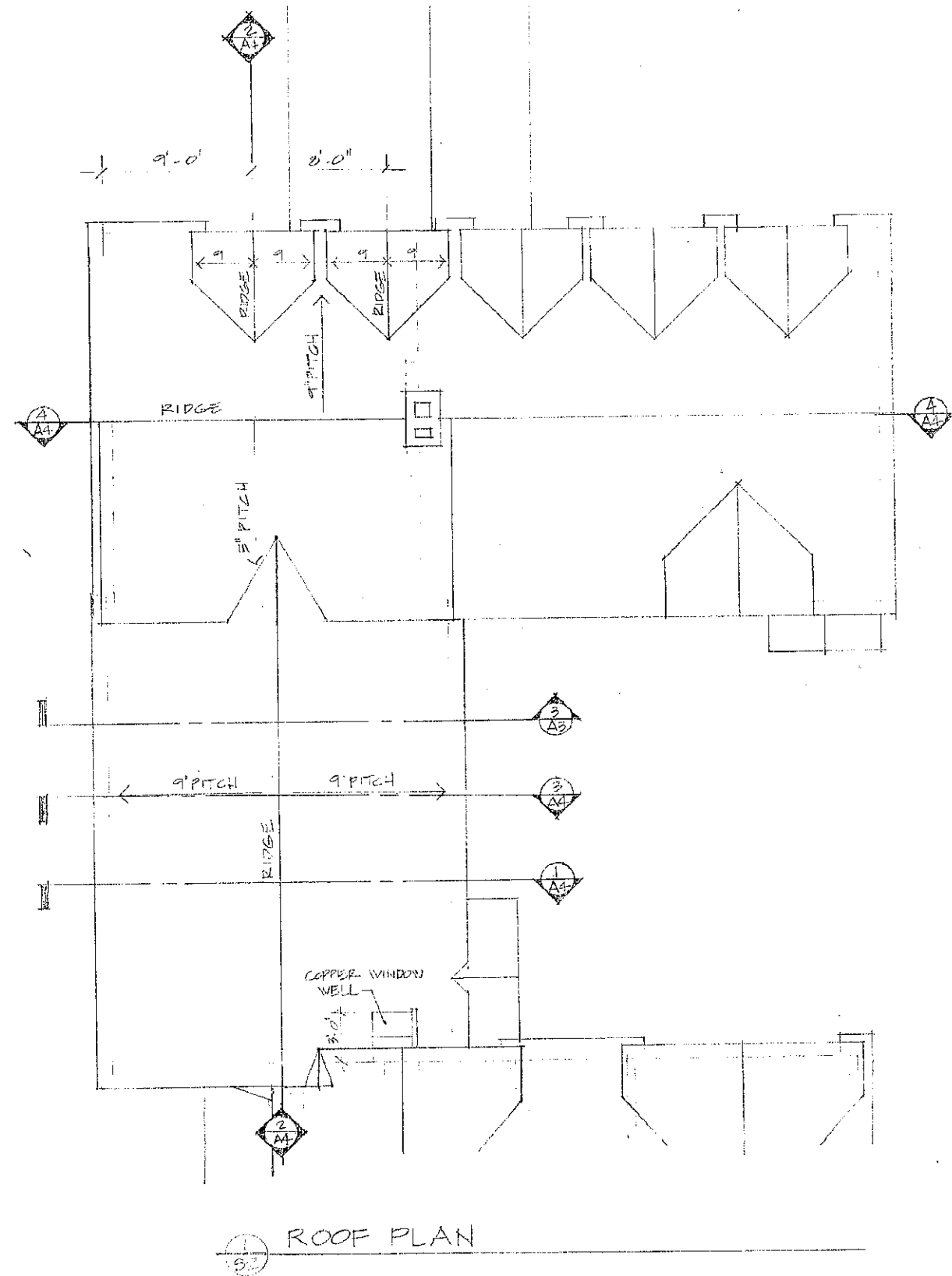
DOOR PLAN

ELECTRICAL OUTLETS PER CODE, LIGHTING & SWITCHING LOCATIONS FOR PRICE CONTRACTOR & ELECTRICIAN TO "WALK-THROUGH" WITH OWNERS TO FINALIZE P

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203 746 5613

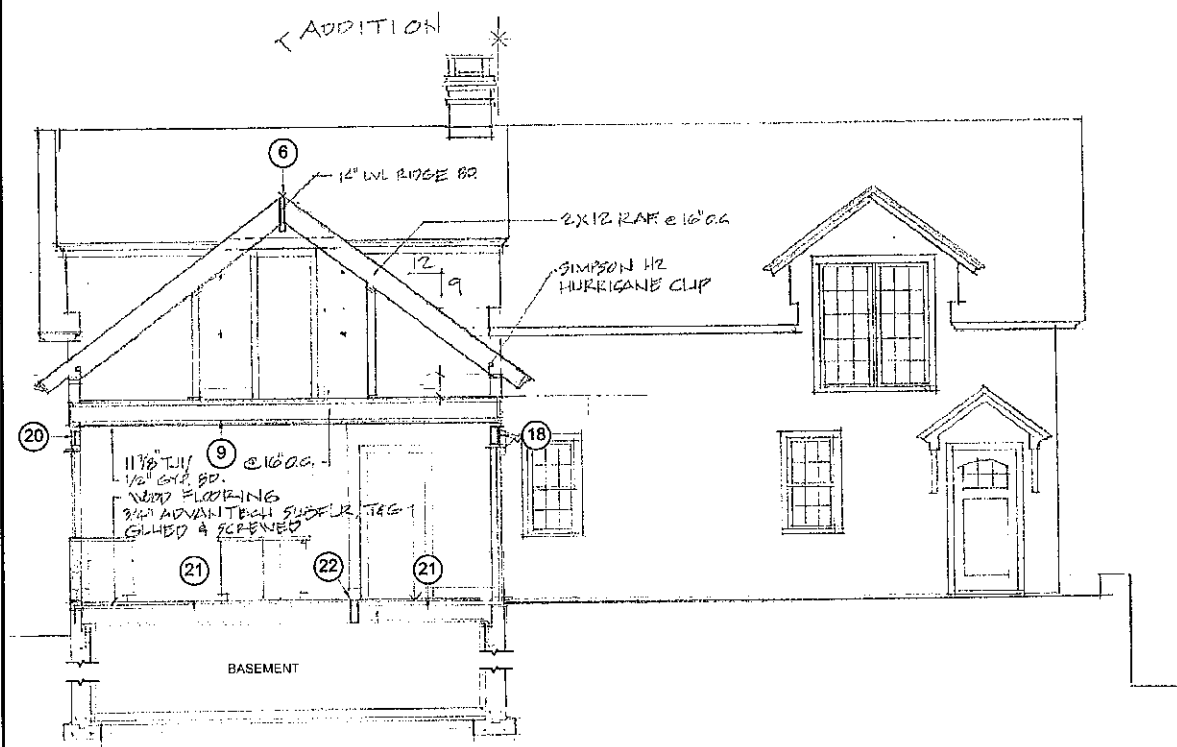
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S



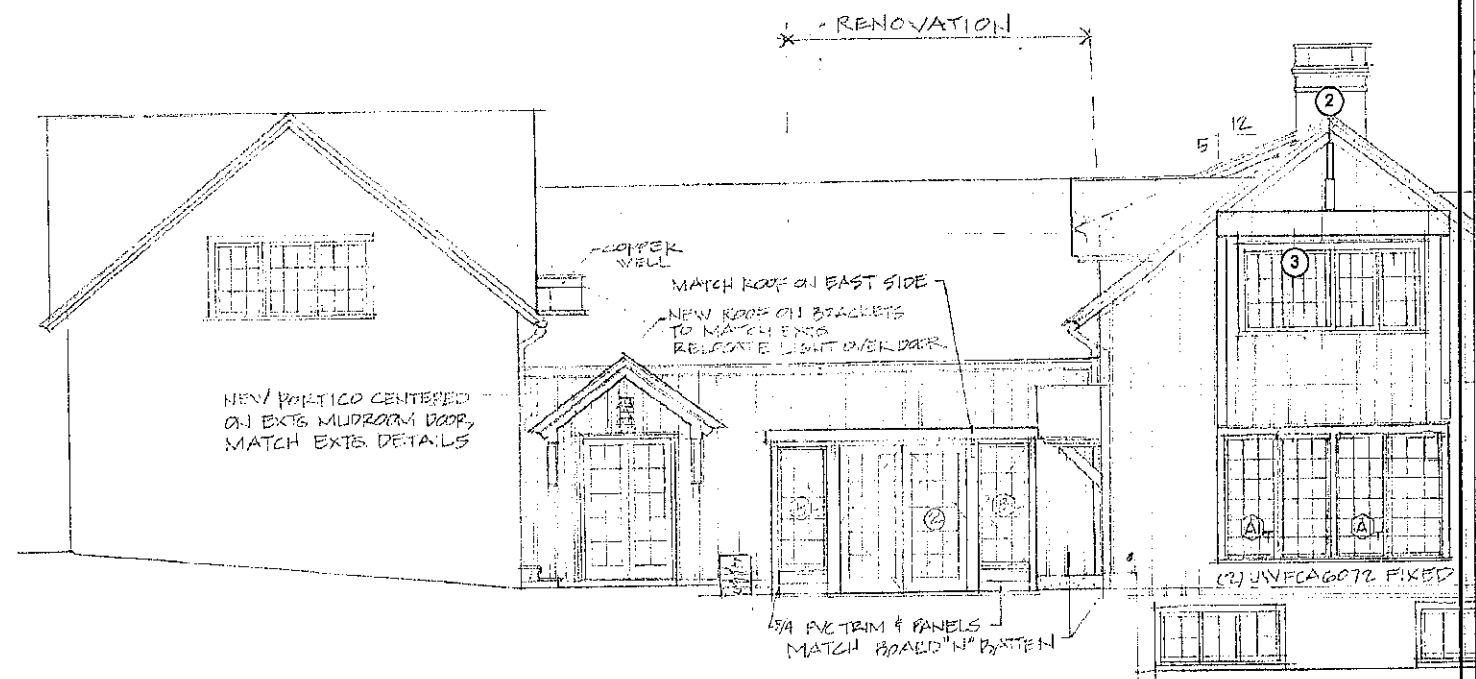
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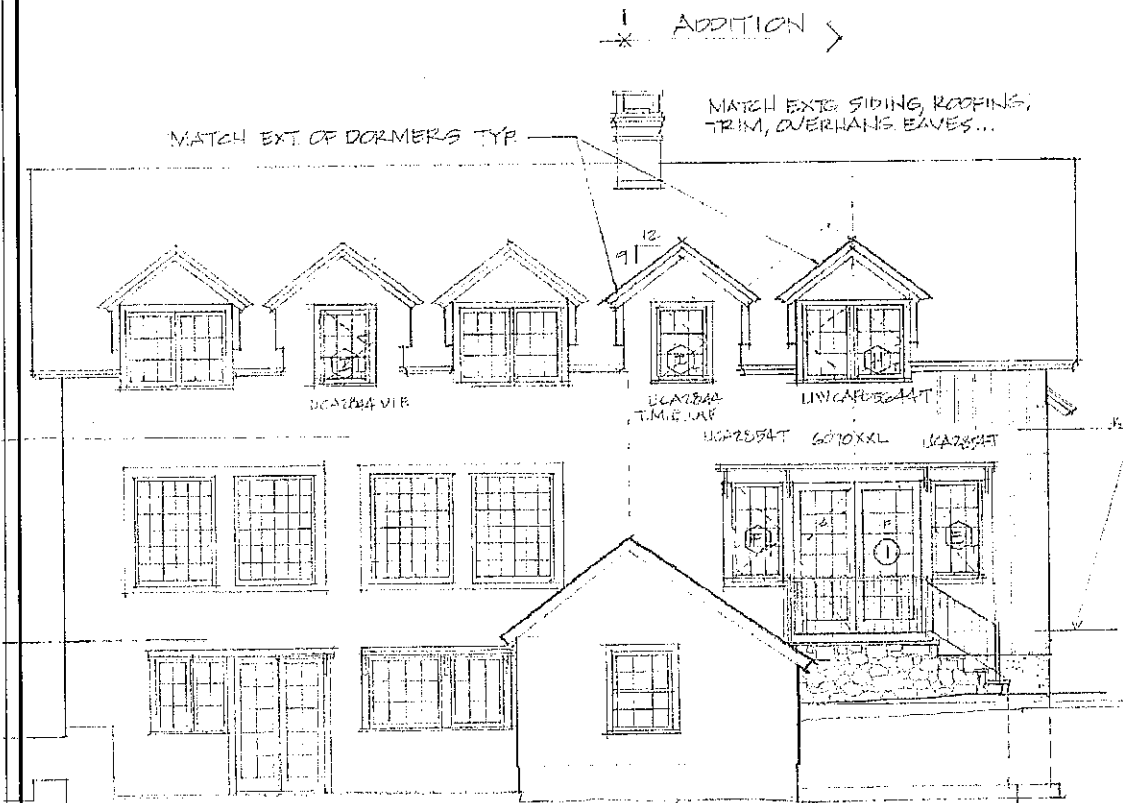


SECTION
SCALE: $\frac{1}{4}'' = 1'-0''$

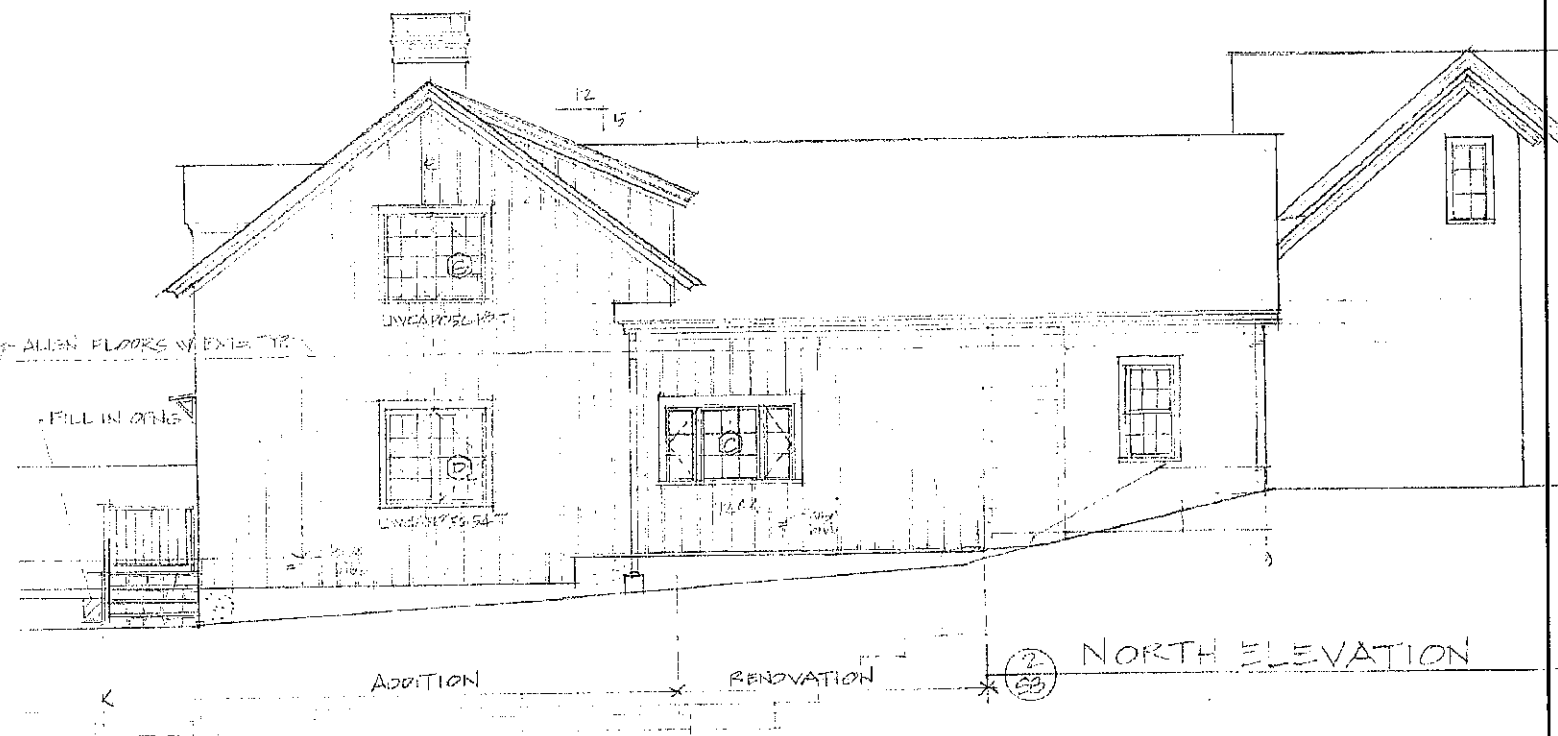
WEST ELEVATION



① SOUTH ELEVATION



93 EAST ELEVATION

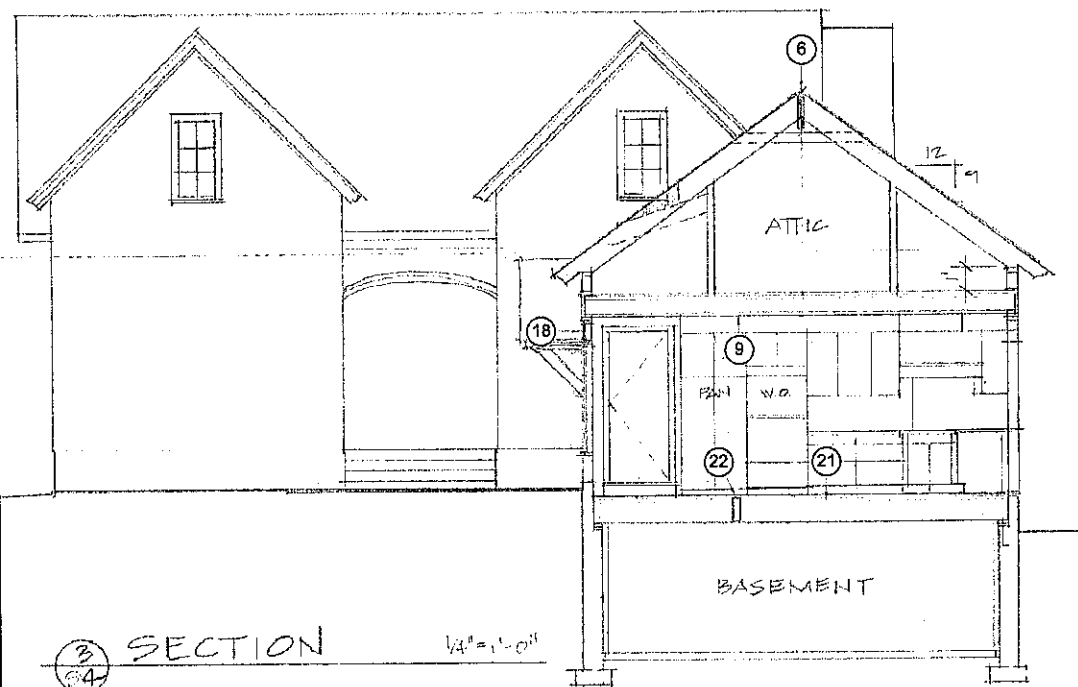


* (2) NORTH ELEVATION

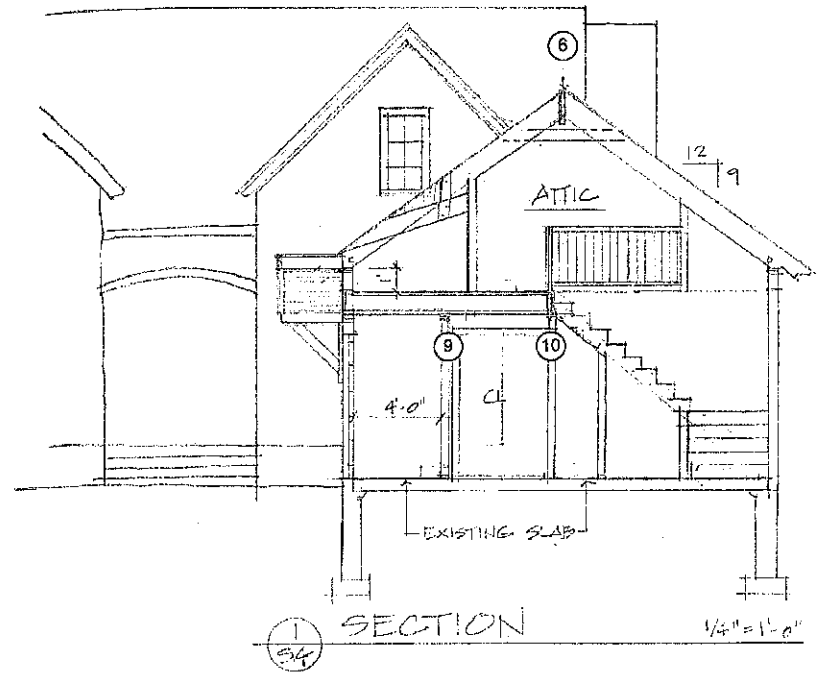
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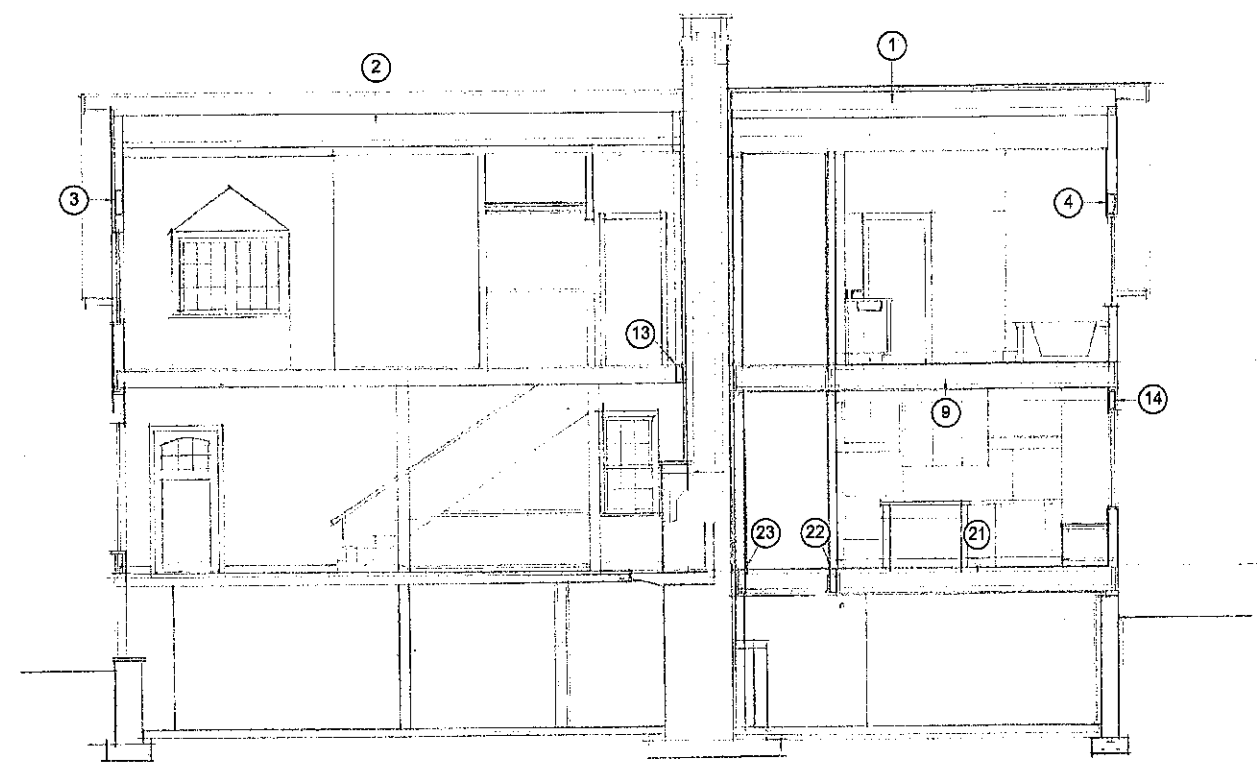
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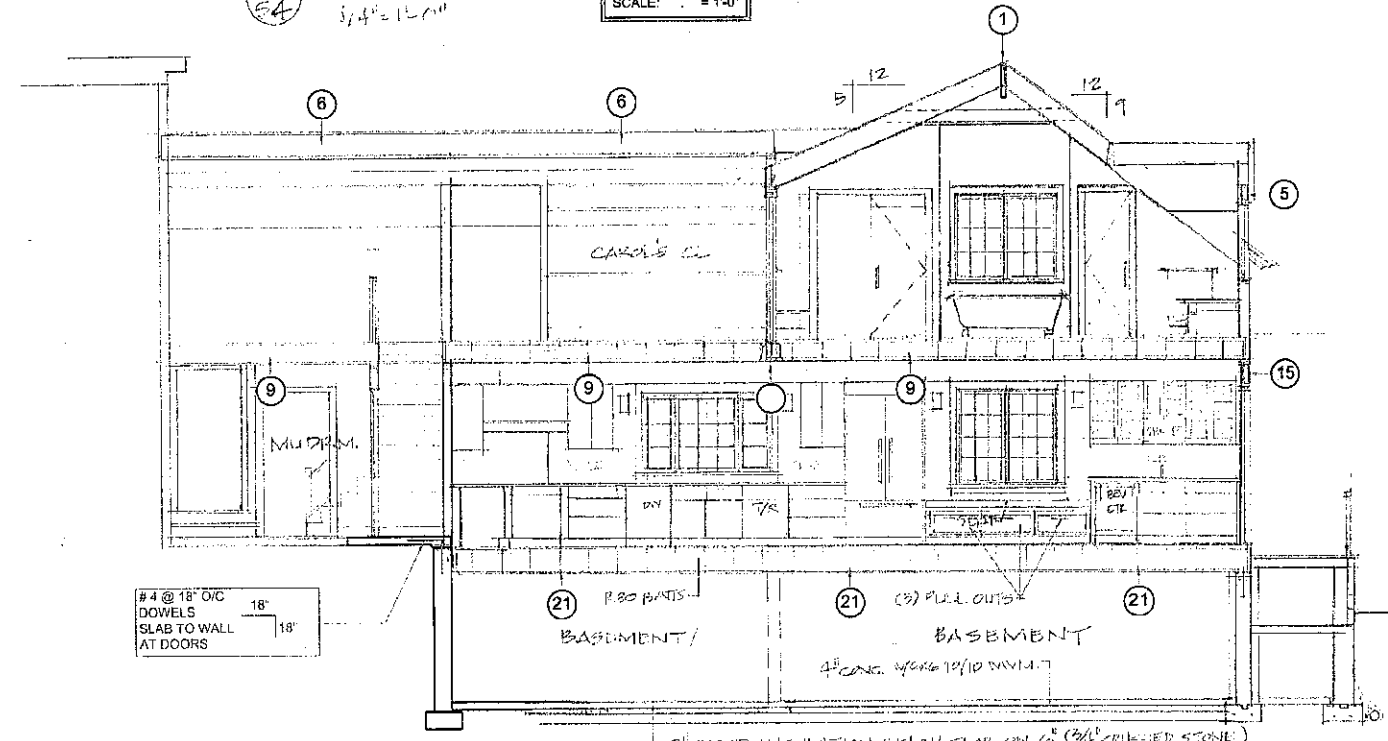
SECTION
SCALE: 1/4" = 1'-0"



SECTION
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SECTION
SCALE: 1/4" = 1'-0"

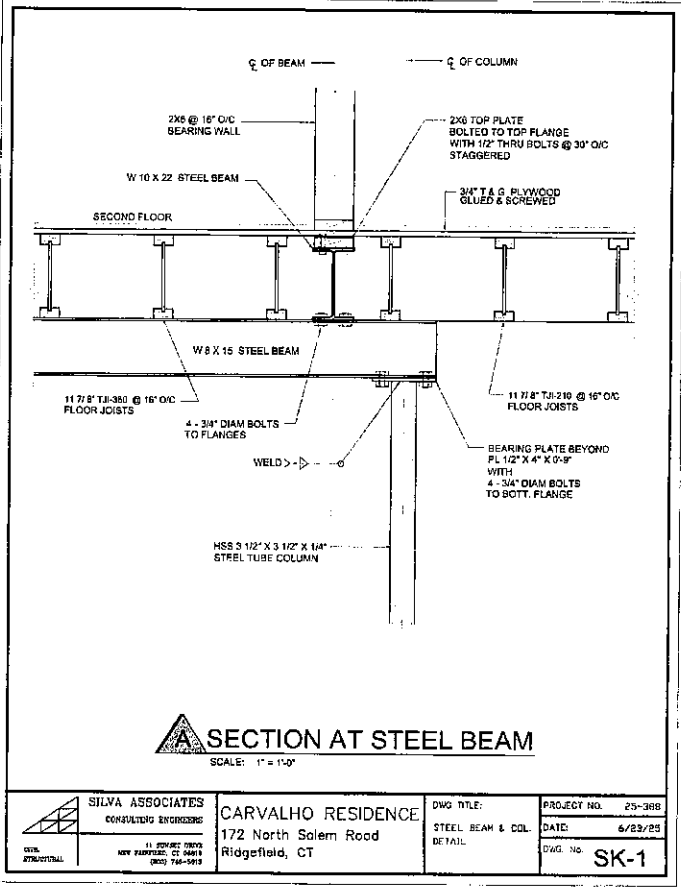


SECTION
SCALE: 1/4" = 1'-0"

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S4



GENERAL NOTES

FOUNDATION

- DESIGN SOIL BEARING CAPACITY IS 4000 PSF AND SHALL BE VERIFIED IN THE FIELD BY ENGINEER WHEN CONSTRUCTION BEGINS.
- NEW FOOTINGS SHALL REST ON UNDISTURBED SOIL. APPROVED BY THE STRUCTURAL ENGINEER.
- EXCAVATION SHALL BE PROTECTED FROM FROST DURING COLD WEATHER.
- DO NOT UNDERMINE OR DAMAGE EXISTING FOUNDATION WALLS. REPORT IMMEDIATELY IF SUCH CONDITION IS LIKELY TO BE CREATED.
- EXISTING FOUNDATION WALLS SHALL BE REPAIRED AS NEEDED OR AS SHOWN ON PLANS.
- WALL CONSTRUCTION JOINTS: SPACE NOT MORE THAN 30 FEET o.c. SECTIONS OF WALLS SHALL BE POURED ALTERNATELY TO MINIMIZE SHRINKAGE.
- SHELVES ON FOUNDATION WALLS: REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS AND DETAILS NOT SHOWN ON STRUCTURAL DRAWINGS. IF THERE IS A QUESTION OR DISCREPANCY CONTACT THE ARCHITECT.

STRUCTURAL CONCRETE

- CONCRETE STRENGTH: 3000 PSI AT 28 DAYS.
- ROD REINFORCEMENT: ASTM A615 GRADE 60. (GRADE 40 MAY BE USED FOR TIES AND STRUPTIES).
- MESH REINFORCEMENT: ASTM A185 AND A82.
- DETAILS WORKMANSHIP AND PROCEDURE: ACI 318 AND ACI 315.
- LOCATION OF CONSTRUCTION JOINTS AND SPLICES FOR STEEL REINFORCEMENT SHALL BE APPROVED BY THE ENGINEER. MINIMUM SPLICE SHALL BE 40 BAR DIAMETERS FOR ALL REBAR.
- SLABS ON GRADE: 4" THICK REINFORCED WITH 5/8" - 10"10 WELDED WIRE FABRIC. PLACE SLABS ON VAPOR BARRIER AND ON 6" MIN OF COMPACTED GRAVEL. BACKFILL UNDER SLABS ON GRADE SHALL BE COMPACTED THOROUGHLY WITH GRANULAR CLEAN FILL IN LAYERS OF 12" MAX. A 2" SLAB MAY BE USED IN CRAWL SPACES.
- CALCIUM CHLORIDE, OR ANY ADMIXTURE CONTAINING CHLORINE SALTS, SHALL NOT BE USED IN ANY CONCRETE.
- ALL CONCRETE EXPOSED TO WEATHER SHALL BE AIR-ENTRAINED.
- ALL REINFORCING STEEL SHALL BE INSPECTED BY THE STRUCTURAL ENGINEER OR REPRESENTATIVE, BEFORE CONCRETE IS POURED.

STRUCTURAL STEEL

- MATERIAL: ASTM A36, FOR ROLLED STEEL SECTIONS. ASTM A500, GRADE B, FOR COLD FORMED STEEL TUBING.
- WORKMANSHIP: AISC "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS," LATEST EDITION.
- CONNECTIONS: SHOP AND FIELD CONNECTIONS CAN BE WELDED OR BOLTED WITH A325 H.S. BOLTS. SLEEVES AND LINTELS MAY BE MACHINE BOLTED IN FIELD.
- WELDING: PERFORMED BY CERTIFIED OPERATORS AS PER AMERICAN WELDING SOCIETY STANDARD QUALIFICATION PROCEDURE.
- COLUMNS ARE TO BE PLUMBED, GUYPED AND BRACED WITH TEMPORARY BRACES UNTIL BEAMS ARE BOLTED IN PLACE.
- SUBMIT SHOP DRAWINGS FOR ALL STRUCTURAL STEEL WORK FOR REVIEW BEFORE FABRICATION.

STRUCTURAL LUMBER

- ALL STRUCTURAL LUMBER WORK SHALL CONFORM WITH "NFA" NATIONAL DESIGN SPECIFICATIONS, AND THE STANDARDS OF AITC.
- SAWN LUMBER: SOUTHERN PINE OR DOUGLAS FIR STRUCTURAL GRADE #2. MOISTURE CONTENT FOR LUMBER SHALL NOT EXCEED 19%.
- LAMINATED VENEER LUMBER (LVL) AND PARALLEL STRAND LUMBER (PSL): SHALL BE "MICROLAM" AND "PARALLAM" TYPE BEAMS AND COLUMNS AS MANUFACTURED BY "LEVEL" OR APPROVED EQUAL.
- ENGINEERED JOISTS SHALL BE "TJI / PRO" TYPE AS MANUFACTURED BY "LEVEL" OR APPROVED EQUAL. SIZE AND SPACING AS SHOWN.
- USE GALVANIZED "SIMPSON / STRONG-TIE CONNECTORS" (OR EQUAL) AT ALL FLUSH FRAMING AT ALL RAFTERS TO PLATE HURRICANE WIND CONNECTIONS, AND AS SHOWN ON DRAWINGS.
- ALL NAILING SHALL BE PERFORMED IN STRICT ACCORDANCE WITH THE STATE OF CONNECTICUT BUILDING CODES.

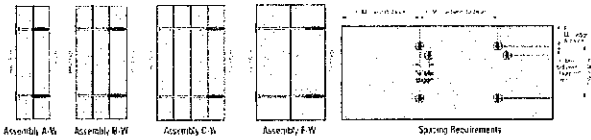
MISCELLANEOUS

- CONTRACTOR SHALL FURNISH AND BE SOLELY RESPONSIBLE FOR ALL TEMPORARY BRACING REQUIRED TO MAINTAIN PLUMBNESS AND STABILITY OF ALL STRUCTURAL ELEMENTS DURING CONSTRUCTION.
- ALL SECTIONS AND DETAILS SHALL BE CONSIDERED TYPICAL AND APPLY FOR THE SAME AND SIMILAR SITUATIONS THROUGHOUT THE BUILDING UNLESS OTHERWISE SPECIFICALLY NOTED.
- THE GENERAL CONTRACTOR SHALL COORDINATE AND BE RESPONSIBLE FOR THE APPROVED SIZE AND LOCATION OF ALL OPENINGS THROUGH ROOF, FLOORS AND WALLS.
- ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH STATE AND LOCAL CODES AND REGULATIONS.

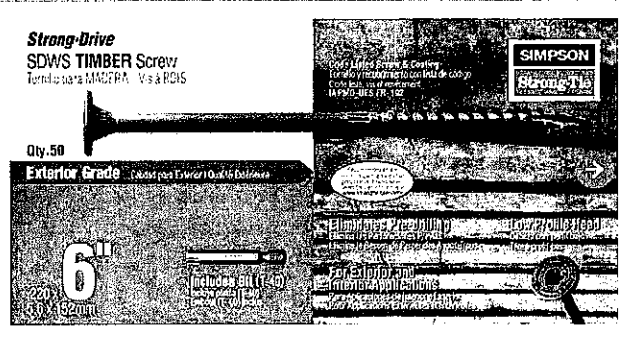
SDW - A General Uniform Code Applied to Other Outside Members
Side Loaded Multi-Ply LVL, PS, and LVL Assemblies

Multiple Members Assembly	Component	Normal Span Length (ft.)	Loaded Side	2 Rows	3 Rows	2 Rows	3 Rows	2 Rows	3 Rows
A-W	2x12 S2	21'	Top	1200	1800	900	1200	600	900
B-W	1 1/2" x 8" S2	4'	End	200	300	150	200	100	150
C-W	1 1/2" x 8" S2	4'	End	200	300	150	200	100	150
D-W	2x12 S2	21'	Top	1200	1800	900	1200	600	900

1. Load applied normal to the face of the member.
2. Lateral bracing provided at the ends of the member and at intermediate points.
3. All connections shall be made in accordance with the provisions of the National Design Specification for Wood Construction, 2015 Edition, and the manufacturer's instructions.



MULTI-PLY LVL BEAM ASSEMBLY DETAILS



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Addition & Renovation
Carvalho Residence 172 North Salem Rd.
Ridgefield CT

6/24/25

N.T.S.

DR BY M

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UNIFORM, LOADED BEAM	BEAM # 1	CLEAR SPAN IN FT.: 14.5 UNIFORM LOAD IN KIPS/FT.: .48	RIDGE BEAM	Options: → 1-1 3/4" X 14" LVL → 2-1 3/4" X 11 7/8" LVL'S	UNIFORM AND CONCENTRATED LOADS	BEAM # 11	CLEAR SPAN IN FT.: 4.5 UNIFORM LOAD IN KIPS/FT.: .85	BEAM # 16	CLEAR SPAN IN FT.: 9.5 UNIFORM LOAD IN KIPS/FT.: 1.28	GIRDER
REACTION= 3.48 KIPS A(109) = 47.8991 IN 2 A(160) = 32.6250 IN 2	Options: → 1-1 3/4" X 14" LVL → 2-1 3/4" X 11 7/8" LVL'S	MOMENT = 12.615 K-FT S(24) = 6.3075 IN 3 S(1250) = 121.104 IN 3 S(1450) = 104.4 IN 3 S(1800) = 84.1 IN 3 I(29)360/240 = 34.0605 / 22.707 IN 4 I(1.5)360/240 = 658.503 / 439.002 IN 4 I(1.7)360/240 = 581.032 / 387.3547 IN 4	Options: → 1-1 3/4" X 14" LVL → 2-1 3/4" X 11 7/8" LVL'S	REACTION LEFT = 1.642436 KIPS REACTION RIGHT = 1.677564 KIPS A(109) = 23.08575 IN 2 A(160) = 15.72716 IN 2 MOMENT = 11.42436 K-FT S(24) = 5.712179 IN 3 S(1250) = 109.6738 IN 3 S(1450) = 94.54641 IN 3 S(1800) = 76.16238 IN 3 I(29)360/240 = 41.46224 / 27.65483 IN 4 I(1.5)360/240 = 801.9901 / 534.6601 IN 4 I(1.7)360/240 = 707.6383 / 471.7588 IN 4 I(1.9)360/240 = 633.15 / 422.1 IN 4	Options: → 2-1 3/4" X 11 7/8" LVL'S → 2-1 3/4" X 14" LVL'S	REACTION= 1.9125 KIPS A(109) = 26.31881 IN 2 A(160) = 17.92969 IN 2	HEADER VERIFY EXISTING MIN. SIZES: → 2-2X8 → 3-2X6 → 2-1 3/4" X 5 1/2" LVL'S	REACTION= 6.08 KIPS A(109) = 83.66933 IN 2 A(160) = 57 IN 2	Options: → 3-1 3/4" X 9 1/2" LVL'S → 2-1 3/4" X 11 7/8" LVL'S	
MOMENT = 10.9375 K-FT S(24) = 5.46875 IN 3 S(1250) = 105 IN 3 S(1450) = 90.5172 IN 3 S(1800) = 72.9166 IN 3 I(29)360/240 = 25.45797 / 16.9719 IN 4 I(1.5)360/240 = 492.1875 / 328.125 IN 4 I(1.7)360/240 = 434.2831 / 289.522 IN 4								MOMENT = 14.44 K-FT S(24) = 7.22 IN 3 S(1250) = 138.624 IN 3 S(1450) = 119.503 IN 3 S(1800) = 96.26667 IN 3 I(29)360/240 = 25.5486 / 17.02924 IN 4 I(1.5)360/240 = 34.8531 / 23.2368 IN 4 I(1.7)360/240 = 30.75469 / 20.50313 IN 4		
UNIFORM, LOADED BEAM	BEAM # 2	CLEAR SPAN IN FT.: 7.5 UNIFORM LOAD IN KIPS/FT.: .36 POINT LOAD P1 IN KIPS: 3.48 DIST. OF P1 FROM LEFT SUPPORT IN FT.: 3.5 POINT LOAD P2 IN KIPS: 0	HEADER	Options: → 2-1 3/4" X 7 1/4" LVL'S → 2-2X10 → 2-1 3/4" X 20" LVL'S	UNIFORM, LOADED BEAM	BEAM # 12	CLEAR SPAN IN FT.: 3 UNIFORM LOAD IN KIPS/FT.: .85	BEAM # 17	CLEAR SPAN IN FT.: 7.5 UNIFORM LOAD IN KIPS/FT.: .12 POINT LOAD P1 IN KIPS: 1.47 DIST. OF P1 FROM LEFT SUPPORT IN FT.: 4.5 POINT LOAD P2 IN KIPS: 0	FLUSH BEAM
REACTION= 7.14 KIPS A(109) = 98.25688 IN 2 A(160) = 66.9375 IN 2	Options: → 3-1 3/4" X 18" LVL'S → 2-1 3/4" X 20" LVL'S	MOMENT = 45.5175 K-FT S(24) = 22.75875 IN 3 S(1250) = 436.968 IN 3 S(1450) = 376.696 IN 3 S(1800) = 303.45 IN 3 I(29)360/240 = 216.1297 / 144.0864 IN 4 I(1.5)360/240 = 4178.507 / 2785.671 IN 4 I(1.7)360/240 = 3686.918 / 2457.945 IN 4	Options: → 2-1 3/4" X 7 1/4" LVL'S → 2-2X10 → 2-1 3/4" X 20" LVL'S	REACTION LEFT = 1.944 KIPS REACTION RIGHT = 3.336 KIPS A(109) = 45.90826 IN 2 A(160) = 31.375 IN 2 MOMENT = 4.599 K-FT S(24) = 2.2995 IN 3 S(1250) = 44.1504 IN 3 S(1450) = 36.96669 IN 3 S(1800) = 30.66 IN 3 I(29)360/240 = 4.281828 / 2.854552 IN 4 I(1.5)360/240 = 82.782 / 55.188 IN 4 I(1.7)360/240 = 73.04294 / 48.6953 IN 4 I(1.9)360/240 = 65.35421 / 43.56948 IN 4	Options: → 2-1 3/4" X 11 7/8" LVL'S → 2-1 3/4" X 9 1/2" LVL'S	REACTION= 1.275 KIPS A(109) = 17.54587 IN 2 A(160) = 11.95313 IN 2	HEADER VERIFY EXISTING MIN. SIZES: → 2-2X6 → 3-2X4	REACTION LEFT = 1.075 KIPS REACTION RIGHT = 1.332 KIPS A(109) = 18.33028 IN 2 A(160) = 12.4875 IN 2 MOMENT = 1.456 K-FT S(24) = 1.728 IN 3 S(1250) = 33.1776 IN 3 S(1450) = 28.60136 IN 3 S(1800) = 23.04 IN 3 I(29)360/240 = 4.826483 / 3.217656 IN 4 I(1.5)360/240 = 93.31207 / 62.28901 IN 4 I(1.7)360/240 = 82.33413 / 54.58942 IN 4 I(1.9)360/240 = 73.66738 / 49.11159 IN 4	Options: → 2-1 3/4" X 9 1/2" LVL'S → 2-1 3/4" X 11 7/8" LVL'S	
MOMENT = 45.5175 K-FT S(24) = 22.75875 IN 3 S(1250) = 436.968 IN 3 S(1450) = 376.696 IN 3 S(1800) = 303.45 IN 3 I(29)360/240 = 216.1297 / 144.0864 IN 4 I(1.5)360/240 = 4178.507 / 2785.671 IN 4 I(1.7)360/240 = 3686.918 / 2457.945 IN 4										
UNIFORM AND CONCENTRATED LOADS	BEAM # 3	CLEAR SPAN IN FT.: 13 UNIFORM LOAD IN KIPS/FT.: .08	FLOOR JOISTS LL=40 DL=20	Options: → 11 7/8" TJI-210 @ 16" O.C. → 9 1/2" TJI-230 @ 16" O.C.	UNIFORM AND CONCENTRATED LOADS	BEAM # 13	CLEAR SPAN IN FT.: 7.5 UNIFORM LOAD IN KIPS/FT.: .48 POINT LOAD P1 IN KIPS: 7.1 DIST. OF P1 FROM LEFT SUPPORT IN FT.: 3.75 POINT LOAD P2 IN KIPS: 0	BEAM # 18	CLEAR SPAN IN FT.: 12.25 UNIFORM LOAD IN KIPS/FT.: .85	TRANSFER HEADER UNDER POST FROM RIDGE
REACTION LEFT = 3.99 KIPS REACTION RIGHT = 3.99 KIPS A(109) = 54.0826 IN 2 A(160) = 37.40625 IN 2 MOMENT = 19.845 K-FT S(24) = 9.922499 IN 3 S(1250) = 190.512 IN 3 S(1450) = 164.2345 IN 3 S(1800) = 132.3 IN 3 I(29)360/240 = 38.80091 / 25.86093 IN 4 I(1.5)360/240 = 750.141 / 500.094 IN 4 I(1.7)360/240 = 661.8991 / 441.2594 IN 4 I(1.9)360/240 = 592.2165 / 394.811 IN 4	Options: → 2-1 3/4" X 11 7/8" LVL'S → 3-1 3/4" X 11 1/4" LVL'S	MOMENT = 1.69 K-FT S(24) = .845 IN 3 S(1250) = 16.224 IN 3 S(1450) = 13.98621 IN 3 S(1800) = 11.26667 IN 3 I(29)360/240 = 4.000965 / 2.72731 IN 4 I(1.5)360/240 = 79.092 / 52.728 IN 4 I(1.7)360/240 = 69.78706 / 46.52471 IN 4	Options: → 11 7/8" TJI-210 @ 16" O.C. → 9 1/2" TJI-230 @ 16" O.C.	REACTION LEFT = 6.1 KIPS REACTION RIGHT = 6.1 KIPS A(109) = 83.94496 IN 2 A(160) = 57.1875 IN 2 MOMENT = 18.09375 K-FT S(24) = 9.046875 IN 3 S(1250) = 175.7 IN 3 S(1450) = 149.7414 IN 3 S(1800) = 120.625 IN 3 I(29)360/240 = 25.76886 / 16.8459 IN 4 I(1.5)360/240 = 488.5312 / 325.6875 IN 4 I(1.7)360/240 = 431.057 / 287.3713 IN 4 I(1.9)360/240 = 385.6825 / 257.1217 IN 4	Options: → 2-1 3/4" X 11 7/8" LVL'S → 2-1 3/4" X 9 1/2" LVL'S	REACTION= 5.20625 KIPS A(109) = 71.43655 IN 2 A(160) = 48.8066 IN 2	Options: → 2-1 3/4" X 14" LVL'S → 3-1 3/4" X 9 1/2" LVL'S → PL 12" X 9" FLITCH	MOMENT = 15.94414 K-FT S(24) = 7.97071 IN 3 S(1250) = 153.0038 IN 3 S(1450) = 131.9515 IN 3 S(1800) = 106.2943 IN 3 I(29)360/240 = 36.36913 / 24.24609 IN 4 I(1.5)360/240 = 703.1366 / 468.7577 IN 4 I(1.7)360/240 = 620.4146 / 413.6097 IN 4		
UNIFORM AND CONCENTRATED LOADS	BEAM # 4	CLEAR SPAN IN FT.: 19.5 UNIFORM LOAD IN KIPS/FT.: .08	FLOOR JOISTS LL=30 DL=20	Options: → 11 7/8" TJI-360 @ 16" O.C. → 14" TJI-210 @ 16" O.C.	UNIFORM, LOADED BEAM	BEAM # 14	CLEAR SPAN IN FT.: 5 UNIFORM LOAD IN KIPS/FT.: .68	BEAM # 19	CLEAR SPAN IN FT.: 19.5 UNIFORM LOAD IN KIPS/FT.: .44 POINT LOAD P1 IN KIPS: 2 DIST. OF P1 FROM LEFT SUPPORT IN FT.: 2.6 POINT LOAD P2 IN KIPS: 3.3 DIST. OF P2 FROM LEFT SUPPORT IN FT.: 7.1 POINT LOAD P3 IN KIPS: 0	KITCHEN BEAM
REACTION= 1.78 KIPS A(109) = 10.73395 IN 2 A(160) = 7.3125 IN 2	Options: → 2-1 3/4" X 7 1/4" LVL'S → 2-2X10 → 3-2X8	MOMENT = 3.8025 K-FT S(24) = 1.90125 IN 3 S(1250) = 36.504 IN 3 S(1450) = 31.46896 IN 3 S(1800) = 25.35 IN 3 I(29)360/240 = 13.90701 / 9.204671 IN 4 I(1.5)360/240 = 266.9355 / 177.957 IN 4 I(1.7)360/240 = 235.5313 / 157.0209 IN 4	Options: → 11 7/8" TJI-360 @ 16" O.C. → 14" TJI-210 @ 16" O.C.	REACTION= 1.7 KIPS A(109) = 23.3945 IN 2 A(160) = 15.9375 IN 2	Options: → 2-1 3/4" X 7 1/4" LVL'S → 3-1 3/4" X 5 1/2" LVL'S → 2-2X8	REACTION LEFT = 7.113077 KIPS REACTION RIGHT = 6.769923 KIPS A(109) = 97.88638 IN 2 A(160) = 66.6851 IN 2 MOMENT = 41.62385 K-FT S(24) = 20.81193 IN 3 S(1250) = 399.589 IN 3 S(1450) = 344.4733 IN 3 S(1800) = 277.4924 IN 3 I(29)360/240 = 151.1377 / 100.7584 IN 4 I(1.5)360/240 = 2921.995 / 1947.996 IN 4 I(1.7)360/240 = 2578.231 / 1718.82 IN 4 I(1.9)360/240 = 2308.838 / 1537.892 IN 4	Options: → W 10 X 22 → W 8 X 28	MOMENT = 15.94414 K-FT S(24) = 7.97071 IN 3 S(1250) = 153.0038 IN 3 S(1450) = 131.9515 IN 3 S(1800) = 106.2943 IN 3 I(29)360/240 = 36.36913 / 24.24609 IN 4 I(1.5)360/240 = 703.1366 / 468.7577 IN 4 I(1.7)360/240 = 620.4146 / 413.6097 IN 4		
UNIFORM AND CONCENTRATED LOADS	BEAM # 5	CLEAR SPAN IN FT.: 5.5 UNIFORM LOAD IN KIPS/FT.: .5	FLUSH BEAM AT STAIRS	Options: → 2-1 3/4" X 11 7/8" LVL'S → 1-1 3/4" X 14" LVL'S	UNIFORM, LOADED BEAM	BEAM # 15	CLEAR SPAN IN FT.: 6.5 UNIFORM LOAD IN KIPS/FT.: .6	BEAM # 20	CLEAR SPAN IN FT.: 7 UNIFORM LOAD IN KIPS/FT.: .85 POINT LOAD P1 IN KIPS: 7.1 DIST. OF P1 FROM LEFT SUPPORT IN FT.: 1 POINT LOAD P2 IN KIPS: 0	BEAM (DROPPED) TRANSFER HEADER
REACTION= 1.375 KIPS A(109) = 18.92702 IN 2 A(160) = 12.89063 IN 2	Options: → 2-1 3/4" X 5 1/2" → 2-2X8	MOMENT = 1.890625 K-FT S(24) = 8453125 IN 3 S(1250) = 18.15 IN 3 S(1450) = 15.64655 IN 3 S(1800) = 12.60417 IN 3 I(29)360/240 = 1.936261 / 1.290841 IN 4 I(1.5)360/240 = 37.43438 / 24.95625 IN 4 I(1.7)360/240 = 33.03053 / 22.02022 IN 4	Options: → 2-1 3/4" X 11 7/8" LVL'S → 1-1 3/4" X 14" LVL'S	REACTION= 1.95 KIPS A(109) = 26.83487 IN 2 A(160) = 18.28125 IN 2	Options: → 2-1 3/4" X 7 1/4" LVL'S → 2-2X10 → 3-2X8 → 3-1 3/4" X 5 1/2" LVL'S	REACTION LEFT = 9.666715 KIPS REACTION RIGHT = 3.989286 KIPS A(109) = 124.6887 IN 2 A(160) = 84.6442 IN 2 MOMENT = 9.761413 K-FT STL	Options: → W 10 X 15 → W 8 X 15 → W 6 X 16S11	MOMENT = 1.71875 K-FT S(24) = 859375 IN 3 S(1250) = 16.5 IN 3 S(1450) = 14.22414 IN 3 S(1800) = 11.45833 IN 3 I(29)360/240 = 1.600216 / 1.06681 IN 4 I(1.5)360/240 = 30.9375 / 20.625 IN 4 I(1.7)360/240 = 27.2978 / 18.19853 IN 4		
UNIFORM, LOADED BEAM	BEAM # 6	CLEAR SPAN IN FT.: 5.5 UNIFORM LOAD IN KIPS/FT.: .5	FLUSH BEAM AT STAIRS	Options: → 2-1 3/4" X 11 7/8" LVL'S → 1-1 3/4" X 14" LVL'S	UNIFORM, LOADED BEAM	BEAM # 16	CLEAR SPAN IN FT.: 7 UNIFORM LOAD IN KIPS/FT.: .85 POINT LOAD P1 IN KIPS: 7.1 DIST. OF P1 FROM LEFT SUPPORT IN FT.: 1 POINT LOAD P2 IN KIPS: 0	BEAM # 21	CLEAR SPAN IN FT.: 13 UNIFORM LOAD IN KIPS/FT.: .08	FLOOR JOISTS LL=40 DL=20
REACTION= 1.375 KIPS A(109) = 18.92702 IN 2 A(160) = 12.89063 IN 2	Options: → 2-1 3/4" X 5 1/2" → 2-2X8	MOMENT = 1.890625 K-FT S(24) = 8453125 IN 3 S(1250) = 18.15 IN 3 S(1450) = 15.64655 IN 3 S(1800) = 12.60417 IN 3 I(29)360/240 = 1.936261 / 1.290841 IN 4 I(1.5)360/240 = 37.43438 / 24.95625 IN 4 I(1.7)360/240 = 33.03053 / 22.02022 IN 4	Options: → 2-1 3/4" X 11 7/8" LVL'S → 1-1 3/4" X 14" LVL'S	REACTION= 1.95 KIPS A(109) = 26.83487 IN 2 A(160) = 18.28125 IN 2	Options: → 2-1 3/4" X 7 1/4" LVL'S → 2-2X10 → 3-2X8 → 3-1 3/4" X 5 1/2" LVL'S	REACTION LEFT = 9.666715 KIPS REACTION RIGHT = 3.989286 KIPS A(109) = 124.6887 IN 2 A(160) = 84.6442 IN 2 MOMENT = 9.761413 K-FT STL	Options: → W 10 X 15 → W 8 X 15 → W 6 X 16S11	MOMENT = 1.71875 K-FT S(24) = 859375 IN 3 S(1250) = 16.5 IN 3 S(1450) = 14.22414 IN 3 S(1800) = 11.45833 IN 3 I(29)360/240 = 1.600216 / 1.06681 IN 4 I(1.5)360/240 = 30.9375 / 20.625 IN 4 I(1.7)360/240 = 27.2978 / 18.19853 IN 4		
UNIFORM, LOADED BEAM	BEAM # 7	CLEAR SPAN IN FT.: 5.5 UNIFORM LOAD IN KIPS/FT.: .5	FLUSH BEAM AT STAIRS	Options: → 2-1 3/4" X 11 7/8" LVL'S → 1-1 3/4" X 14" LVL'S	UNIFORM, LOADED BEAM	BEAM # 17	CLEAR SPAN IN FT.: 7 UNIFORM LOAD IN KIPS/FT.: .85 POINT LOAD P1 IN KIPS: 7.1 DIST. OF P1 FROM LEFT SUPPORT IN FT.: 1 POINT LOAD P2 IN KIPS: 0	BEAM # 22	CLEAR SPAN IN FT.: 13 UNIFORM LOAD IN KIPS/FT.: .08	FLOOR JOISTS LL=40 DL=20
REACTION= 1.375 KIPS A(109) = 18.92702 IN 2 A(160) = 12.89063 IN 2	Options: → 2-1 3/4" X 5 1/2" → 2-2X8	MOMENT = 1.890625 K-FT S(24) = 8453125 IN 3 S(1250) = 18.15 IN 3 S(1450) = 15.64655 IN 3 S(1800) = 12.60417 IN 3 I(29)360/240 = 1.936261 / 1.290841 IN 4 I(1.5)360/240 = 37.43438 / 24.95625 IN 4 I(1.7)360/240 = 33.03053 / 22.02022 IN 4	Options: → 2-1 3/4" X 11 7/8" LVL'S → 1-1 3/4" X 14" LVL'S	REACTION= 1.95 KIPS A(109) = 26.83487 IN 2 A(160) = 18.28125 IN 2	Options: → 2-1 3/4" X 7 1/4" LVL'S → 2-2X10 → 3-2X8 → 3-1 3/4" X 5 1/2" LVL'S	REACTION LEFT = 9.666715 KIPS REACTION RIGHT = 3.989286 KIPS A(109) = 124.6887 IN 2 A(160) = 84.6442 IN 2 MOMENT = 9.761413 K-FT STL	Options: → W 10 X 15 → W 8 X 15 → W 6 X 16S11	MOMENT = 1.71875 K-FT S(24) = 859375 IN 3 S(1250) = 16.5 IN 3 S(1450) = 14.22414 IN 3 S(1800) = 11.45833 IN 3 I(29)360/240 = 1.600216 / 1.06681 IN 4 I(1.5)360/240 = 30.9375 / 20.625 IN 4 I(1.7)360/240 = 27.2978 / 18.19853 IN 4		
UNIFORM, LOADED BEAM	BEAM # 8	CLEAR SPAN IN FT.: 5.5 UNIFORM LOAD IN KIPS/FT.: .5	FLUSH BEAM AT STAIRS	Options: → 2-1 3/4" X 11 7/8" LVL'S → 1-1 3/4" X 14" LVL'S	UNIFORM, LOADED BEAM	BEAM # 18	CLEAR SPAN IN FT.: 7 UNIFORM LOAD IN KIPS/FT.: .85 POINT LOAD P1 IN KIPS: 7.1 DIST. OF P1 FROM LEFT SUPPORT IN FT.: 1 POINT LOAD P2 IN KIPS: 0	BEAM # 23	CLEAR SPAN IN FT.: 13 UNIFORM LOAD IN KIPS/FT.: .08	FLOOR JOISTS LL=40 DL=20
REACTION= 1.375 KIPS A(109) = 18.92702 IN 2 A(160) = 12.89063 IN 2	Options: → 2-1									

Silva Associates P.E. Structural Engineer
11 Sunset Drive New Fairfield CT 06812
203 746 5613

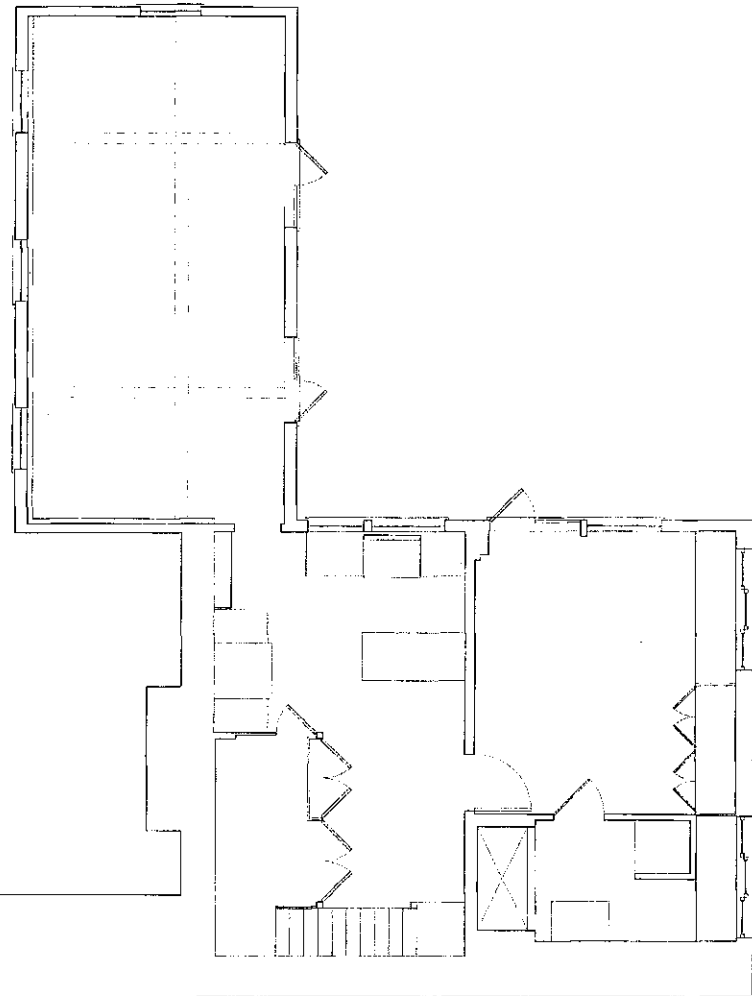
Addition & Renovation
Carvalho Residence 172 North Salem Rd.
Ridgfield CT

6/24/25

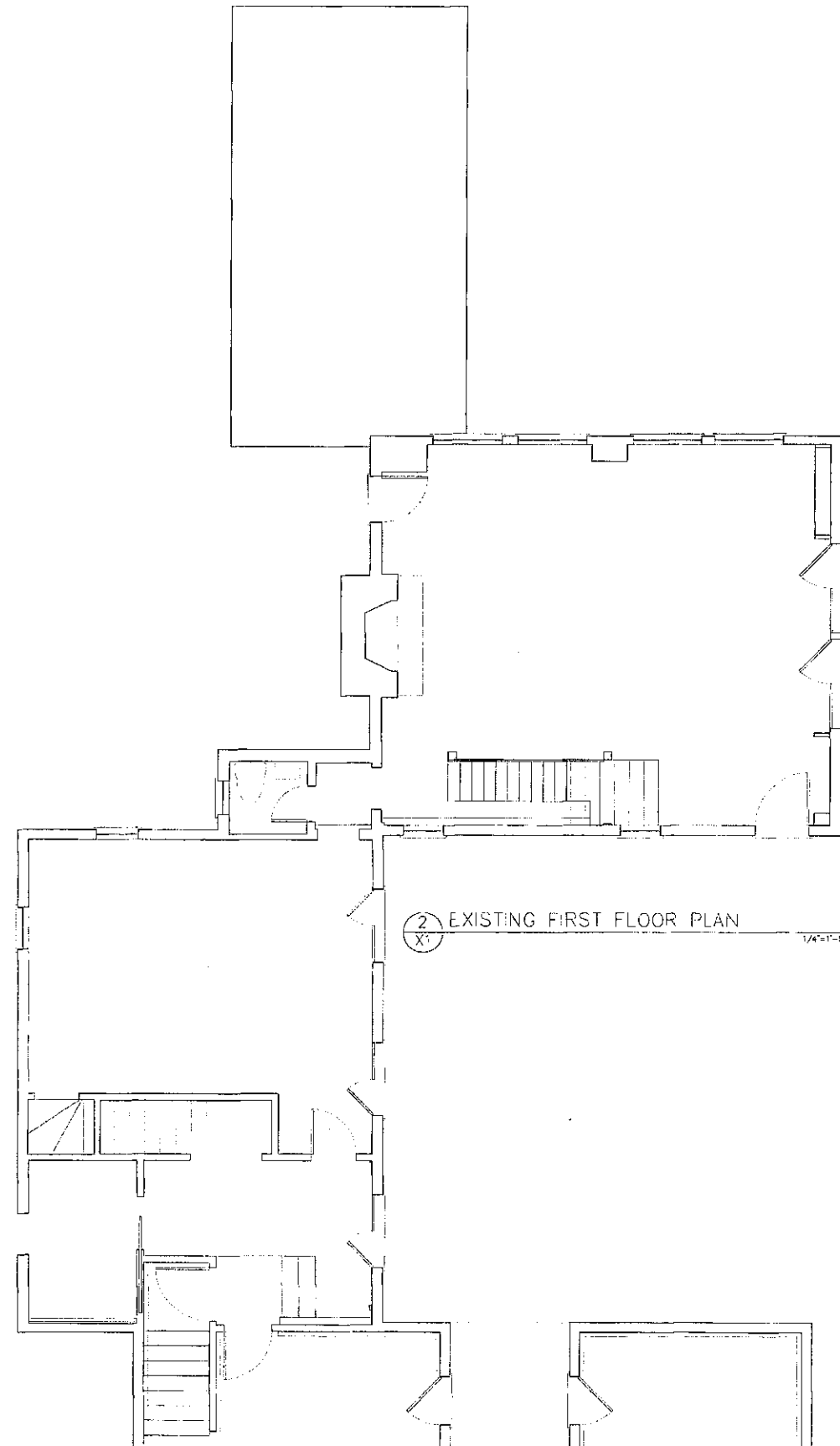
N.T.S.

OK [Signature]

S5



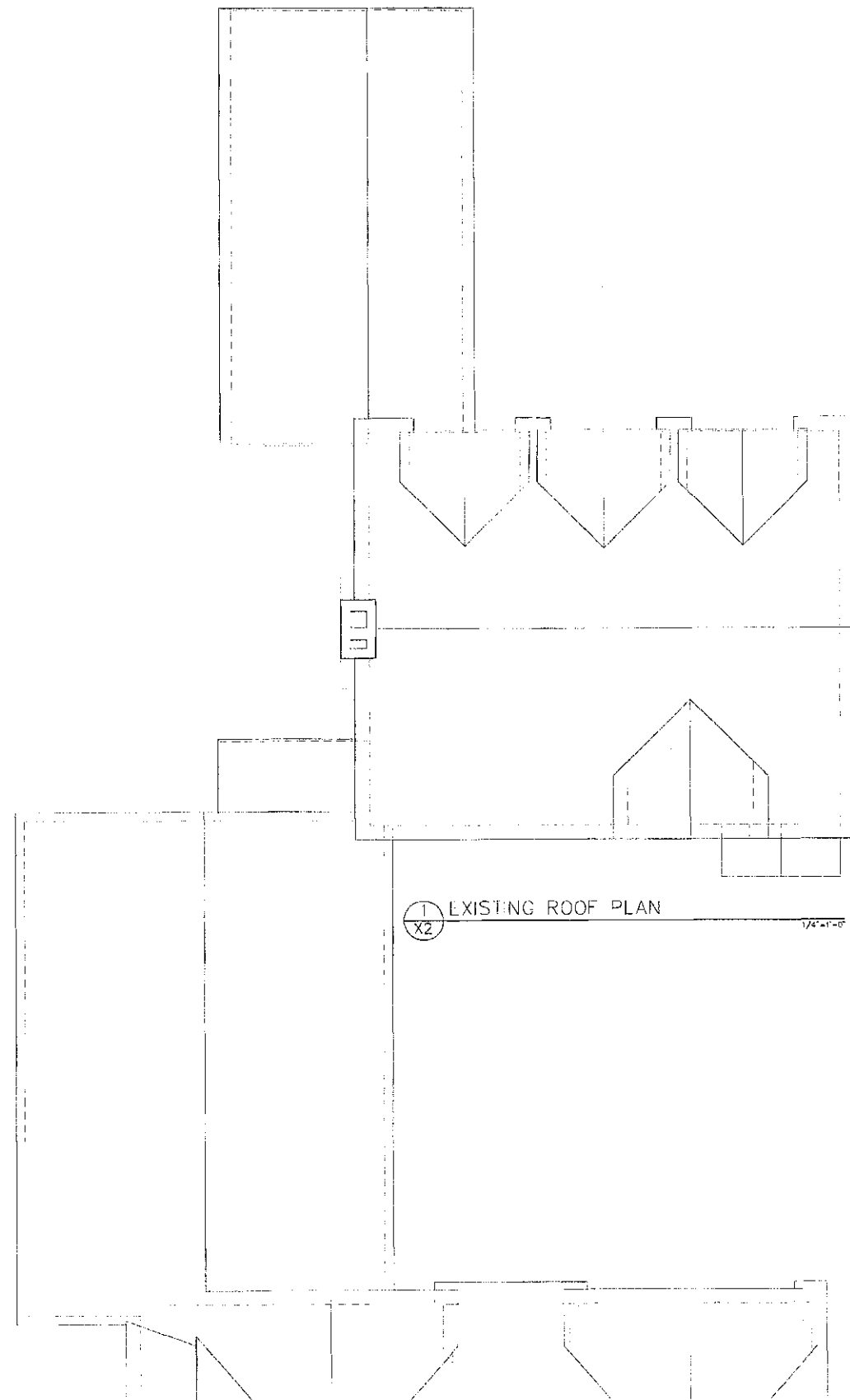
1
X1
EXISTING LOWER LEVEL FLOOR PLAN
1/4"=1'-0"



2
X1
EXISTING FIRST FLOOR PLAN
1/4"=1'-0"

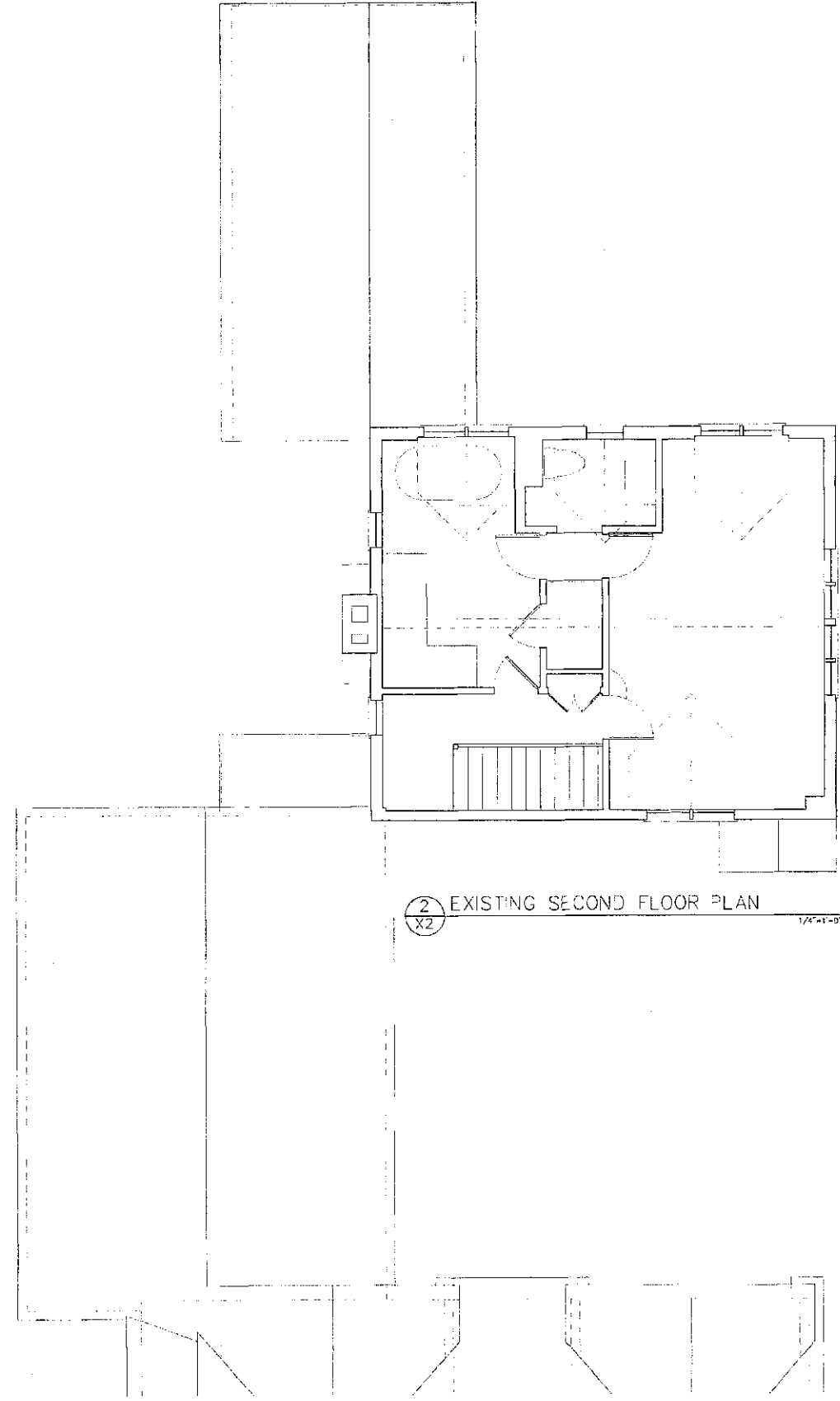
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MICHAEL LOBUGLIO A.I.A ARCHITECT			EXISTING CARVALHO RESIDENCE 172 NORTH SALEM ROAD RIDGEFIELD, CT	
82 TAUNTON LAKE RD., NEWTOWN, CT TEL. (203) 512-0353			EXISTING LOWER LEVEL & FIRST FLOOR PLANS SCALE: 1/4"=1'-0"	

X-1



1 EXISTING ROOF PLAN

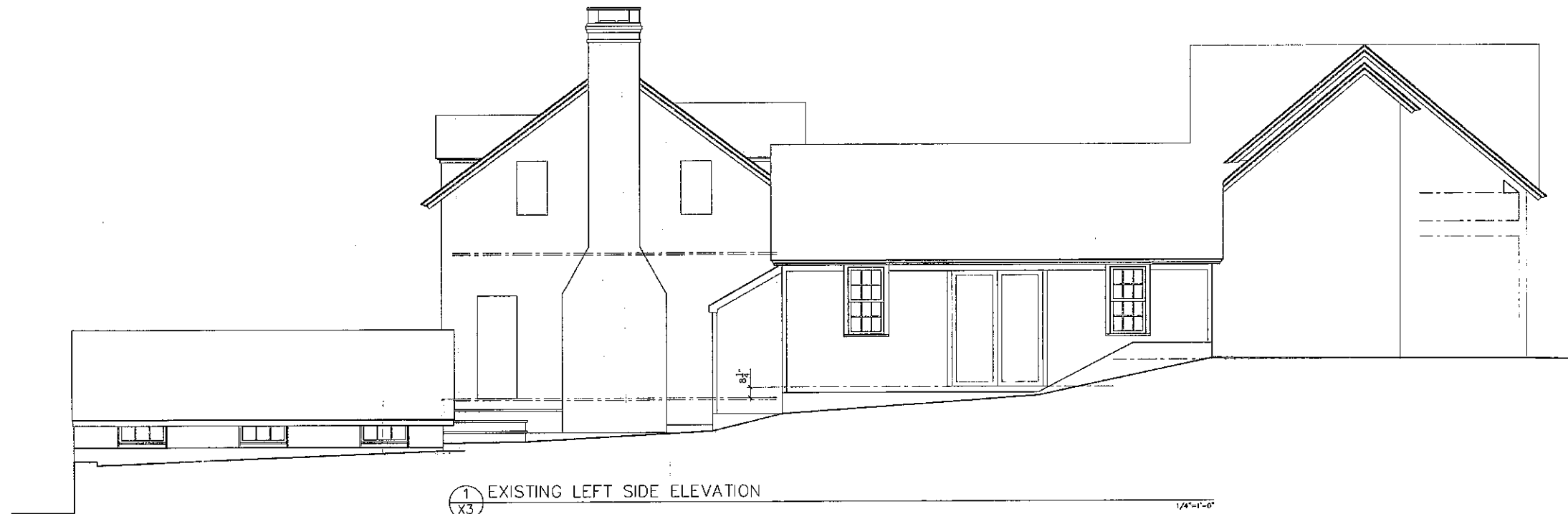
1/4"=1'-0"



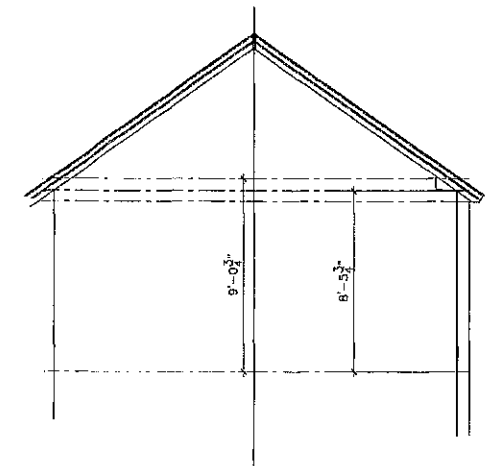
2 EXISTING SECOND FLOOR PLAN

1/4"=1'-0"

DRAWN BY GJM	CHECKED BY	APPROVED BY	FILE NAME EXISTING.DWG	DATE 2/27/2025	X-2
MICHAEL LOBUGLIO A.I.A ARCHITECT			EXISTING CARVALHO RESIDENCE 172 NORTH SALEM ROAD RIDGEFIELD, CT		
62 TAUNTON LAKE RD., NEWTOWN, CT TEL. (203) 512-0353			EXISTING SECOND FLOOR & ROOF PLANS SCALE: 1/4"=1'-0"		



DRAWN BY GJM	CHECKED BY	APPROVED BY	FILE NAME EXISTING.DWG	DATE 3/4/2025	X-3
MICHAEL LOBUGLIO A.I.A ARCHITECT			EXISTING CARVALHO RESIDENCE 172 NORTH SALEM ROAD RIDGEFIELD, CT		
62 TAUNTON LAKE RD., NEWTOWN, CT TEL. (203) 512-0353			EXISTING ELEVATIONS SCALE: 1/4"=1'-0"		



DRAWN BY GJM	CHECKED BY	APPROVED BY	FILE NAME EXISTING.DWG	DATE 3/3/2025	X-4
MICHAEL LOBUGLIO A.I.A ARCHITECT			EXISTING CARVALHO RESIDENCE 172 NORTH SALEM ROAD RIDGEFIELD, CT		
62 TAUNTON LAKE RD., NEWTOWN, CT TEL. (203) 512-0353			EXISTING ELEVATIONS SCALE: 1/4"=1'-0"		